

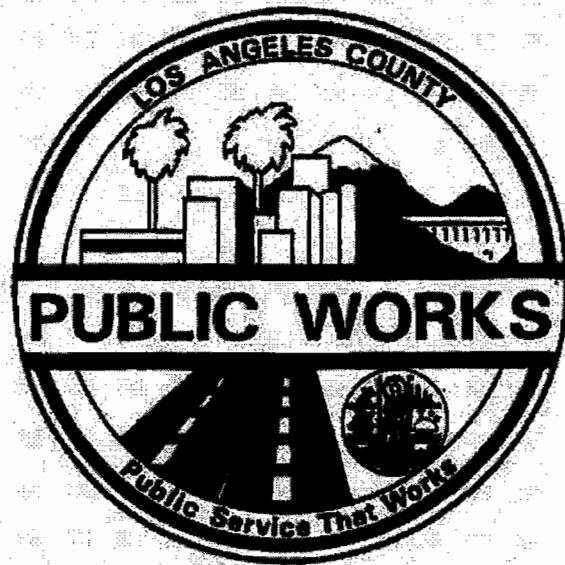
County of Los Angeles

Tentative Tract Map No. 053653

Regional Hydrology Study

Final Draft

June 2006



COUNTY OF LOS ANGELES -- TENTATIVE TRACT MAP NO. 053653

Regional Hydrology Study

Prepared for:



Lyons Canyon, LLC

Prepared by:

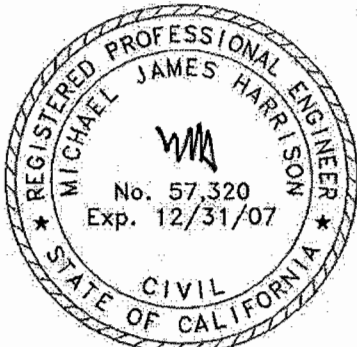
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Preface



6/8/06

Prepared by

A handwritten signature in black ink, appearing to be "MJH", written over a horizontal line.

Mike Harrison, P.E.
RCE #57,320, Expires: December 31, 2007



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Nomenclature

'	feet	in	inch
"	inch	L_o	overland flow path length
<	less than	MAP	Mean annual precipitation
>	greater than	max	maximum
ac	acre	MEP	maximum extent practicable
ac-ft	acre - feet	mi	mile
APN	County Assessor's parcel number	min	minimum
ARC	antecedent runoff condition	misc	miscellaneous
BMPs	best management practices	msl	mean sea level
C	Rational Method runoff coefficient	MWC	municipal water company
Caltrans	California Department of Transportation	MWD	municipal water district
CDMG	California Division of Mines & Geology	NPDES	National Pollutant Discharge Elimination System
cfs	cubic feet per second	NRCS	National Resource Conservation Service
City	City of Santa Clarita	o.d.	outside diameter
CMP	corrugated metal pipe	O&M	Operations and maintenance
CN	SCS curve number	ped.	Pedestrian
Chl	open channel	Q	flow quantity
Consultant	Diamond West Engineering	Qty	quantity
County	County of Los Angeles	R.C.E.	California, Registered Civil Engineer
C_p	pan coefficient	RCP	reinforced concrete pipe
d/s	downstream	req'd	required
DWR	California Department of Water Resources	RWQCB	California Regional Water Quality Control Board
E	evaporation	s	second
EGL	energy grade line	SCS	Soil Conservation Service
FEMA	Federal Emergency Management Agency	sf	square feet
FIP	Finance and Implementation Plan	SUSMP	County, Standard Urban Storm Water Mitigation Plan
FIRM	Flood Insurance Rate Map	t_c	storm duration (time of concentration)
FIS	Flood Insurance Study	t_p	time from start of storm to peak runoff
ft	feet	t_r	rain storm duration
ft/s	feet per second	T	transmissivity
g	acceleration due to gravity	TR-20	SCS Technical Release Number 20
gpm	U.S. gallons per minute	TR-55	SCS Technical Release Number 55
gpd	U.S. gallons per day	UMP	Urban Management Plan
gpd/ft ²	U.S. gallons per day per square foot	u/s	upstream
H	total hydraulic head	USACE	U.S. Army Corps of Engineers
h	horizontal	USEPA	U.S. Environmental Protection Agency
HEC	Hydrologic Engineering Center	USGS	U.S. Geological Survey
HEC-HMS	HEC-HMS Computer Program	V	volume
HEC-RAS	HEC-RAS Computer Program	v	vertical
HGL	hydraulic grade line	w.s.	water surface
hr	hour		
i	rainfall intensity		
i_a	initial abstraction		
i.d.	inside diameter		
imp	impervious		



Executive Summary

The purpose of this report is to facilitate the planning and implementation of drainage infrastructure improvements to accommodate storm water runoff in the general vicinity of the proposed Tentative Tract Map No. 053653.

This report includes an evaluation of existing land use projections in the watershed, existing drainage patterns, alternative storm drainage solutions, potential utility relocation requirements, potential right-of-way issues, and environmental issues. Additionally, this report will identify a lead drainage alternative(s). The results of this report will be the basis for subsequent storm drainage improvements solely for the Tract.

The project (Tract No. 053653) is located on the west side of the City of Santa Clarita roughly between Lyons Avenue, Calgrove Boulevard, and adjacent to and west of the The Old Road (see Figure 1). The project is situated on APN 2826-022-025, -026, -027, & 2826-023-014 which according to the County Assessor contains roughly 232 acres. The vicinity of the detailed study area is the northerly ridge of Towsley Canyon on the south, the southerly ridge of Dewitt Canyon on the north, and The Old Road on the east (see Figure 2 and Appendix 1, Exhibit A). The detailed study area contains roughly 890 acres and is located entirely in the County. The existing land use in the study area contains mainly open space with a small portion of residential and commercial zoning (see Appendix 1, Exhibit B).

This report addresses the impacts from a 2-year and 50-year, 24-hour design storm event. Its intended use is for the development of drainage infrastructure solely by the project.

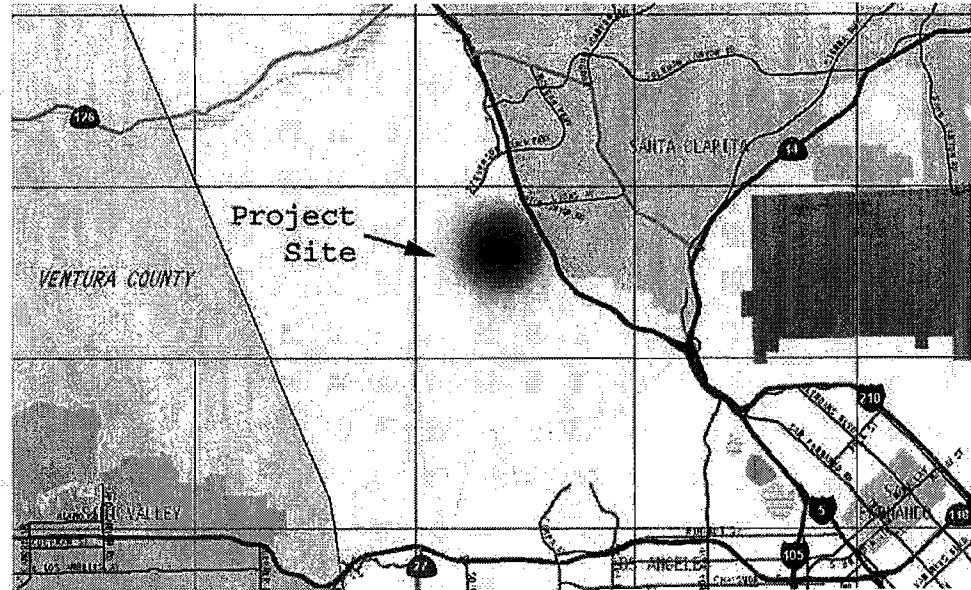
Authorization

This report has been performed at the request of D.R. Horton to redetermine the existing drainage patterns and any storm drainage impacts from the proposed development on the study area. Previous studies by Diamond West Engineering and RBF were for a larger project area. It is not the intent of this report to suggest remediation for any regional drainage issues outside of the project area.



TENTATIVE TRACT MAP NO. 053653 - REGIONAL HYDROLOGY STUDY

**Figure 1.
Regional Location
Map**

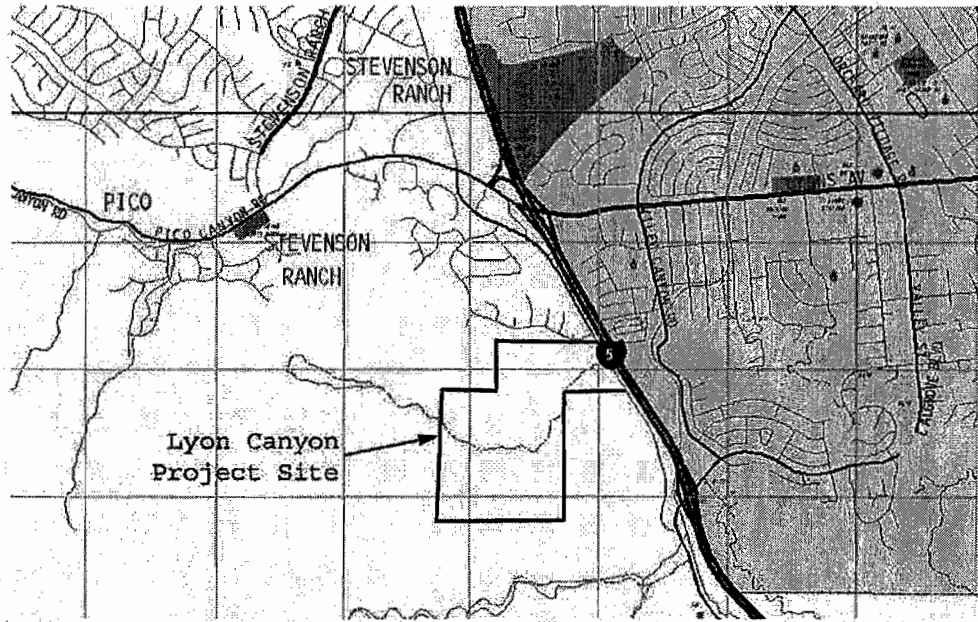


The following information is contained within this report:

1. A description of the existing drainage conditions for the study area.
2. A recommended drainage infrastructure plan showing the locations and sizes of the primary components of the drainage infrastructure that will be needed to accommodate or alleviate storm water runoff generated by the proposed project. See Exhibit J in Appendix 1 and the approved Tentative Map for more information. Drainage infrastructure elements evaluated include:
 - storm drain pipes and/or open channels
 - storm drain culverts
 - storm water impoundments & debris basins (and inlets)
3. Watershed catchment boundaries and hydrologic information that support the drainage infrastructure plan. The County Modified Rational (MODRAT) computer model has been used as the basis for hydrologic evaluations. Discharges expected at numerous key points of concentration have been estimated using the MODRAT computer model for the 50-year, 24-hour storm event.
4. Hydraulic analyses that examine the functional characteristics of the proposed drainage infrastructure. The hydraulic capacities of the proposed storm drains have been evaluated using standard formulas. Volumetric analysis of runoff hydrographs have been evaluated using WMS.
5. Cost estimates for the components of the recommended drainage infrastructure plan(s) are not provided.



Figure 2. Vicinity Map



Study Approach

The Lyons Canyon project was originally submitted to the City of Santa Clarita for review and possible annexation in 2002. Pursuant to the State CEQA Guidelines, the City first circulated a NOP in March 2003. Due to change in project description, another NOP was circulated in December 2003. Subsequently, the project was withdrawn from the City in May 2005. The project area has now been redesigned and entitlement applications were submitted to the County in June 2005. This is the basis for this study report.

The project site is vacant with current on-site structures such as fencing, an abandoned water tank, water wells, and irrigation lines. Other utility structures, such as Southern California Edison electrical distribution lines, are adjacent to or traverse portions of the site.

The project site is currently located within unincorporated Los Angeles County, and is designated as Non-Urban 2 (N2) and Hillside Management (HM) in the Santa Clarita Valley Areawide General Plan, and Non-Urban (R) and Significant Ecological Area (SEA) in the County General Plan. The subject property is zoned as Heavy Agricultural (A-2-2/A-2-1).

The general lay of the land drains from southwest to northeast. Most of the project area drains under The Old Road and Interstate 5 through a double 8-foot by 8-foot box culvert. This culvert ties into a channel that eventually connects to the South Fork of the Santa Clara River. The entire length of the culvert is currently about 75% full of sediment and debris buildup. This existing condition could have adverse drainage impacts on adjacent properties during moderate or even small rainfall events. The



TENTATIVE TRACT MAP NO. 053653 - REGIONAL HYDROLOGY STUDY

results contained herein for existing and proposed capacity of this structure will assume the facility if free and clear of any sediment accumulation.

Both Diamond West Engineering and RBF completed similar hydrologic studies based on previous project designs. Both MORA and MODRAT are acronyms for the Modified Rational Method. They both refer to different sets of hydrologic equations used (and approved) by the County. The previous hydrology reports both used a DOS-based MORA program called FO604.

The MODRAT equations can be found in a program called WMS. WMS is a GIS-based program. The MODRAT parameters and equations are more current than MORA, and WMS is the preferred program at the County.

Summary of objectives and hydrologic conditions

The proposed study area was broken into 48 sub-basins (as shown in Appendix 1, Exhibit G. This includes both on-site and off-site areas. The main objective of this study is to design drainage infrastructure that will not change the historic runoff patterns that are experienced by adjacent properties. With a free discharge and clear of any sediment buildup, the existing double 8-foot by 8-foot culvert has a discharge capacity of about 1,800 cfs without overtopping The Old Road.

Because of the fire hazard in the surrounding area the drainage infrastructure design should consider the effects of burned and bulked runoff from the project area. Drainage facilities should be designed accordingly to accommodate these runoff conditions. Based on conversations with County staff, all debris from the design runoff event is required to be retained on-site. The most feasible way to accomplish this is by installing debris basins according to County design standards.

Two debris basins are planned as shown in Appendix 1, Exhibit J. These basins will retain the debris from the two main branches of off-site run-on to the project area. In addition to the benefits they provide for water quality, they will also serve as combination storm water detention basins. Table 1 shows the difference between existing and proposed runoff conditions (without detention).

Table 1. Pre- vs. Post-Development Runoff

Concentration Point	50-yr, 24-hr (burned & bulked) Flow Rate (cfs)	
	Existing Condition	Proposed Condition
double 8-foot by 8-foot box culvert	1,950	1,425
48-inch CMP culvert	183	120
30-inch CMP culvert	190	190



The proposed roadway grading at the two debris basins is creating a significant amount of storage for storm water runoff. Both basins have a total storage capacity of over 30 acre-feet at the spillway elevations. Assuming an average design rainfall of 8.4-inches and a 50% runoff yield, the runoff from the entire watershed would be about 312 acre-feet. Removing the bulked flow, reducing the potential burned area with the development, and the detaining affects of the debris basins will reduce the runoff for the proposed condition at the double 8-foot by 8-foot culvert from the existing condition. The information shown in this report actually shows an increase in runoff from the project area under burned runoff conditions. This will not occur since the debris basins will act as detention basins. It should also be noted that the existing condition design (burned and bulked) flow rate would inundate The Old Road around the double box culvert.

The purpose of the spillway within each debris basin is to keep it from becoming a jurisdictional dam under the California Division of Safety of Dams (DSOD). The main requirements to remain non-jurisdictional are the storage height needs to be less than 25-feet tall and the potential storage volume is less than 50 acre-feet. The storage height, according to County Public Works, is measured from the spillway elevation to the lowest natural grade on the downstream side of the embankment.

The following items outline the approval process by the DSOD if a basin is determined to be a jurisdictional dam.

- ✓ Submit an application for water rights with the State Water Resources Control Board as required.
- ✓ Hold a pre-application meeting with DSOD to streamline the review process.
- ✓ File an application for the construction of a dam and pay a filing fee.
- ✓ Submit 90% plans and specifications.
- ✓ Submit the environmental documentation as prepared by the lead agency or provide the information required for the DSOD to prepare the environmental documents as the lead agency.
- ✓ Final review and approval of the construction documents.
- ✓ Construct the facilities and obtain a Certificate of Approval from the DSOD.
- ✓ Pay annual fees.

This process could add significant time to the overall project schedule.



Peak Flow Standard

By using the debris basins, the developed runoff from the project area satisfies the peak flow standard. See Appendix 1, Exhibits A and G and Appendix 4 and 5 for the 2-year and 50-year, 24-hour storm events.



Project Setting

The project (Tract No. 053653) is located in and on the west side of the City of Santa Clarita roughly between Lyons Avenue, Calgrove Boulevard, and adjacent to and west of the The Old Road (see Figure 2). The detailed study area contains roughly 890 acres and is located entirely in the County. The existing land use in the study area contains open space and a small portion of residential and commercial zoning (see Appendix 1, Exhibit B).

The existing utilities in the area are a water system, a gas distribution system, an electric service system, and a telephone/fiber optic system. There are production domestic water wells within one mile of the study area. There are no existing storm drainage infrastructure facilities to adequately serve the project area.

**BASIS OF
CONTROL**

The horizontal coordinates shown herein are based on the California Coordinate System of 1983, Zone V in U.S. Survey Feet.

**EXISTING
WATERSHED
CHARACT-
ERISTICS**

The detailed study area consists of approximately 890 acres that is divided into 48 sub-basin watersheds (as shown in Appendix 1, Exhibits A & G). These watersheds are defined by the physical constraints and topographic features that exist and points of interest in the study area. The land uses within the study area consist of open space, residential and commercial zoning. The natural slopes within the sub-basin areas vary roughly from 5% to over 40%.

Storm water runoff generated from the detailed study area generally drains northeasterly as overland flow and as concentrated flow. Concentrated flow generally occurs within the lower elevations. The overland flow from the sub-basins cascades down the respective low points. At each low point, the storm water is either detained, or is further conveyed through downstream sub-basins to the north and east.

The runoff leaves the project area through a series of culverts that cross The Old Road and Interstate 5. A majority of the runoff is conveyed by a double 8-foot by 8-foot box culvert located at the northeast corner of the project area. This culvert is currently about 75% full of sediment and debris buildup and does not have enough discharge capacity to convey the existing design runoff event without flooding The Old Road. The runoff from the project area eventually joins the South Fork of the Santa Clara River.

Flood Insurance Study

The detailed study area is located on the following FEMA FIRM.

Los Angeles County, California (Unincorporated Areas), community panel number 065043 0460 B, December 2, 1980. The detailed study area is located in Zone A and Zone C.



**TENTATIVE TRACT MAP NO. 053653 - REGIONAL HYDROLOGY
STUDY**

Zone C and Zone X are defined as areas of minimal flooding or outside the 500-year floodplain. Zone A is defined as areas of 100-year flood with base flood elevations and factors not determined.

Any construction in Zone A will require a Conditional Letter of Map Revision from FEMA prior to issuance of grading permits. A Letter of Map Revision will be required prior to building occupancy.

Native Soil Properties

The soil types within the study area were identified from the current County Hydrology Manual. Individual soil types are given unique values ranging from 1-180. There are four soil types within the study area, 20, 91, 93, and 97. Soil values can be seen in Appendix 1, Exhibit D.

**EXISTING
GROUNDWATER
CONDITIONS**

The depth to the seasonal high groundwater table is assumed not high enough to be significant. Additional design requirements may be required if it is found to encroach on any new drainage infrastructure, appurtenances, or excavations.



Proposed Drainage Study Approach

The purpose of this hydrology study is to facilitate the planning and implementation of drainage infrastructure improvements to accommodate storm water runoff in the general vicinity of the project area. Additional study objectives include:

- Develop a phased plan that alleviates localized flooding.
- Provide study services consistent with City and County standards.
- Develop phased solutions where the capital improvements can be funded incrementally.
- Develop phased solutions that maximize the cost to benefit ratio.
- Develop solutions that limit O&M costs.
- Develop phased solutions that can fit or be adapted in the ultimate, area-wide solution.
- Involve City and County staff in the development and implementation of the phased solutions.
- Develop phased solutions that will minimize any disturbance to the City, County, and surrounding community.
- Site and operate storm drainage facilities in such a manner that minimizes adverse environmental impacts.

**DESIGN
ANALYSIS**

The approach to design process is to explore a range of solutions. The drainage design presented in this report has been developed based on evaluations of the following constraints:

- Watershed characteristics
- Topography
- Existing land use & its adaptability
- Location of transportation corridors
- Property boundaries & acquisition
- Logical points of drainage outfall
- Agency objectives
- Retrofitting opportunities
- Design level of protection
- Environmental impacts
- Financing (expenses)
- Structure relocation
- Operation and maintenance
- Regulatory compliance
- Agency compliance



- Hydrologic criteria
- Flexibility of service area
- Hydraulic capacities & characteristics

Formulation of the infrastructure design was characterized by an evaluation of all of the above constraints, their level of importance to the successful completion of the project, and their interrelationships with each other.

Debris/Storage Basin Approach

Based on the above mentioned constraints, the proposed design is to develop a series of storm water impoundments to remove accumulated debris and to provide storm water detention to help relieve downstream drainage infrastructure. As requested by the County, the discharge information contained herein does not account for the affects of the detention basins. But it should be recognized that they will exist. These basins need to be designed such that the storage height is less than 25-feet and the storage volume is less than 50 acre-feet to remain as non-jurisdictional dams according to County Public Works and the DSOD.

**UTILITY
CONFLICT
ANALYSIS**

The location of the utilities shown herein is for information only. The location, type, size, and/or depths indicated were obtained from sources of varying reliability. The consultant is not responsible or liable for the accuracy or completeness of those records. All utilities should be field verified as to their actual location, type, size, and depth prior to performing any excavation or other work close to any underground pipeline, conduit, duct, wire, structure or other utilities subject to concerns for safety, displacement, and/or damage by reason of such operations.

The existing utilities in the area are a water system, a gas distribution system, an electric service system, and a telephone/fiber optic system. There are production domestic water wells within one mile of the study area. Ground water monitoring should be considered at any domestic well within one mile of a proposed storm water impoundment. There are no existing storm drainage infrastructure facilities to adequately serve the project area.

For the most part, the drainage collection system has been placed away or adjacent to existing utilities. In reviewing record information, it does not appear that there are any major crossing conflicts. Any conflicts will need to be addressed during the preparation of the construction documents for those facilities.

**RIGHT-OF-
WAY
ANALYSIS**

The property boundaries shown herein are based upon record information. Field verification should be performed during the construction process for any drainage improvements defined herein.

There are no planned right-of-way acquisitions for drainage purposes. Nor are there any planned drainage easements or agreements for the development of the project.



**LAND USE
ANALYSIS**

The County and City General Plan and Zoning Code regulate land use in the study area. Generally, existing land use in the area is consistent with these policy documents. There are no known pending formal applications in the County or City to change land use within the study area. No provisions have been made for changes in future land use within the study area.

**ENVIRONMENT
-AL ANALYSIS**

A Specific Plan EIR is being prepared by others.



Proposed Drainage Description

In order to adequately evaluate the impacts and requirements of the proposed project, the existing drainage conditions were analyzed. Research efforts were made to identify any drainage studies that documented the existing drainage conditions for the study area. The results of these efforts did not find any study that adequately documented those conditions. The purpose of this drainage study is to document the impacts of certain rainfall events on the study area. This information will be the basis of comparison between pre-development and post-development of storm drainage infrastructure improvements.

This proposed drainage description will analyze the effects of the 2-year and 50-year, 24-hour storm events within the study area.

RELATED DOCUMENTS

The Consultant pursued the City, County, and Caltrans for any drainage reports on the study area. The following documents were found which identified potential drainage improvements within the study area.

Drainage Concept / SUSMP Study, 2004

This plan was completed in June 2004 by Diamond West Engineering. The purpose of the study was to evaluate the impacts of the Lyons Canyon Ranch project as it was currently designed. Since that time the project design has changed.

Hydrology and Water Quality Technical Appendix, 2004

This plan was completed in November 2004 by RBF. The purpose of the study was to evaluate the impacts of the Lyons Canyon Ranch project when it was considered to be annexed to the City. Since then the project has been redesigned and it will remain in the County.

METHODOLOGY

Due to the complex nature of the sub-basins, a hydrograph method was chosen to estimate the design storm runoff. The complex aspects of the sub-basins include consideration of available storage and varying times of travel. The Modified Rational Method, as defined in the current County Hydrology Manual was employed to generate the effective runoff within each sub-basin.

The County Hydrology Manual utilizes a Modified Rational Method approach for its hydrologic calculations. The Rational Method is understood to provide peak discharge relative to rainfall intensity. It is not generally preferred in watershed catchments where ponding of storm water occurs. Additionally, it does not typically provide a reasonable relationship between peak storm water discharge and storm water runoff volume. This phenomenon can be seen in Figure 3. As seen on the synthetic rainfall distribution, the County method yields little runoff before or after the peak. This typically produces a sharp, narrow peak, which ultimately requires less storage volume for detention basin analysis.



Because the MODRAT method is considered the 'standard of practice' for this area it will be used to generate the regional hydrology contained herein.

Equation 1. Rational Method

$$Q = CiA$$

Where C = runoff coefficient
i = rainfall intensity (in/hr)
A = drainage area (ac)

Equation 2. Manning Equation

$$V = \frac{1.486}{n} R^{2/3} S^{1/2}$$

Where V = average velocity (ft/s)
n = pipe roughness coefficient
R = hydraulic radius (ft)
S = head loss per unit length of pipe (ft/ft)

The rainfall intensity was taken from County Standards. The runoff coefficient in the rational formula is dependent on the soil type, antecedent moisture condition, recurrence interval, land use, slope, amount of urban development, rainfall intensity, surface and channel roughness, and duration of storm. Equation 3 provides a relationship between all of these factors and was used to calculate the runoff coefficients.

Equation 3. Rational Runoff Coefficient

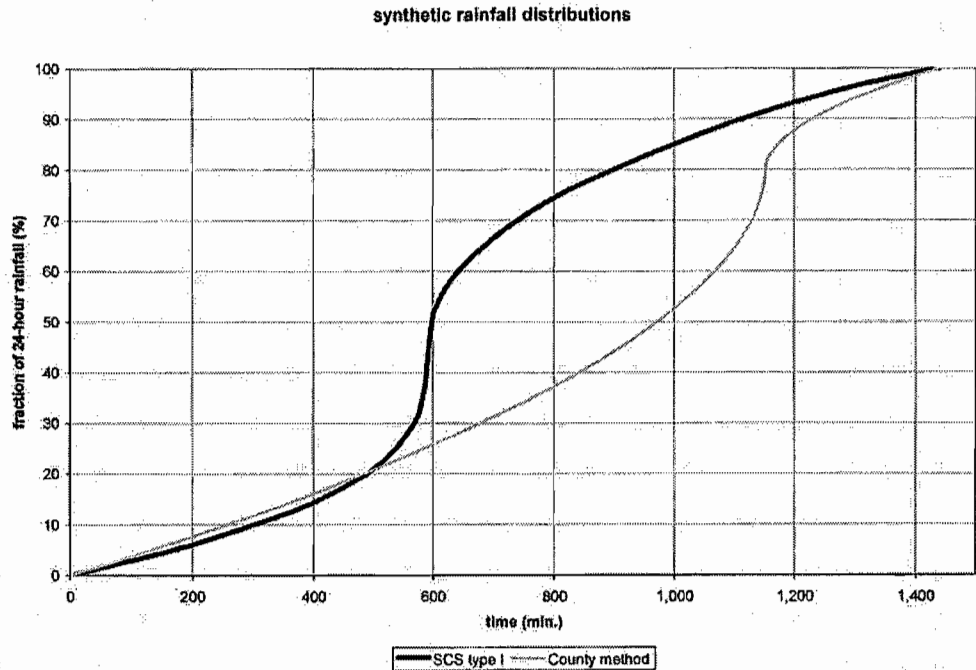
$$C = 7.2(10^{-7})CN^3T^{-0.05} \left[\left((0.01CN)^{0.6} \right) \right]^{-S^{0.2}} (0.001CN^{1.48})^{(0.15-0.11)} [(P+1)/2]^{0.7}$$

Where CN = SCS composite curve number
T = recurrence interval (years)
S = average sub-basin land slope (%)
I = rainfall intensity of recurrence interval (in/hr)
P = percent impervious (decimal)

The average rainfall for the design storm event for the study area per the County Hydrology Manual is about 8.4 inches. With this data and normal antecedent moisture conditions, the runoff yield is roughly 36%. According to County staff a runoff yield analysis is not required with the current hydrologic methods.



Figure 3. synthetic rainfall distribution comparison



Hydrologic Model

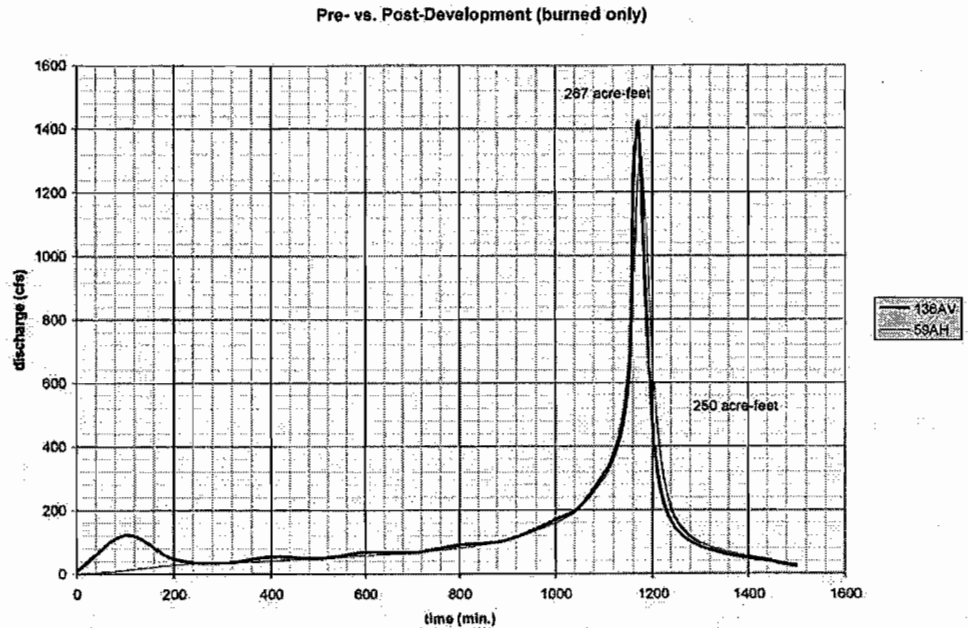
The computer models MODRAT was used to simulate, combine, and route outflow hydrographs within each watershed. The simulation of the hydrologic data is generated by the development of the synthetic unit hydrograph, 50-year design storm pattern, and the runoff hydrograph.

See Figure 4 for a graph of pre-developed vs. post-developed hydrographs at the existing double 8-foot by 8-foot box culvert. The total volume of runoff for both scenarios can be increased 1-3% because the runoff is still occurring at the end of the design storm.

The development of the synthetic unit hydrograph involves the identification of several watershed characteristics including composite curve numbers, soil cover, percent impervious, antecedent moisture conditions, land use, basin area, initial abstractions, hydraulic length, basin slope, and lag time. These parameters are calculated in the following steps:



Figure 4. Pre- v. Post-Development Hydrograph at Double Box Culvert



- The sub-basin watershed boundaries were delineated by WMS on the USGS map.
- Rainfall excess is that part of the total precipitation depth that appears as surface flow during and after a storm event. Rainfall excess equals to total rainfall depth minus losses due to interception by vegetation, infiltration into the soil, and surface depression storage. This process is defined internally in the MODRAT method. The information is based on:
 1. Soil data from the current County Hydrology Manual
 2. Zoning designations in the City and County
- The catchment time of concentration is defined as the time from the center of mass of net rainfall and the center of mass of runoff. The time of concentration for each sub-basin was identified from the County method. This method is shown in equation 4.

Equation 4. Time of Concentration

$$T_c = 10^{-0.507} * (C_d * I)^{-0.519} * L^{0.483} * S^{-0.135}$$

- Where
- T_c = time of concentration in minutes
 - C_d = developed runoff coefficient
 - I = rainfall intensity in inches per hour
 - L = hydrologic length of the catchment in feet
 - S = average watershed land slope in feet per feet



- To adequately define the unit hydrograph, the unit time period of the synthetic critical storm pattern should generally be 30 percent of the basin time of concentration and should use multiples of 1 minute. The unit time period utilized in this report is 1 minute.
- See Appendix catchment soil characteristics, catchment hydrologic characteristics, and hydrograph plots for various locations.

Bulking Analysis

A bulking analysis was performed for the proposed burned runoff conditions by County methods. The entire study area is in debris potential area #3 (see Appendix 1, Exhibit F). Table 2 shows the minimum required size for the debris basins and desilting inlets.

Table 2. Proposed Watershed Debris Analysis

node	area (ac.)	bulking		yield (cy)	note
		factor	rate (cy/sm)		
40A	549	1.48	64,000	54,900	basin #1
115AQ	195	1.52	87,000	26,535	basin #2
131U	6	1.62	140,000	1,315	inlet #8
43F	7	1.62	140,000	1,535	inlet #6
53I	10	1.62	140,000	2,190	inlet #1
79P	10	1.62	140,000	2,190	inlet #2
75O	9	1.62	140,000	1,970	inlet #3
72N	4	1.62	140,000	875	inlet #4
70M	5	1.62	140,000	1,100	inlet #5
140X	4	1.62	140,000	875	inlet #7

The yield results for the inlets should be considered somewhat arbitrary because the curves for Plate 2 and 5 from the County Sedimentation Manual have a minimum drainage area of 64 acres.

Upon final design the debris inlets should be sized according to Table 2.

Flow Routing

Flow routing methods for storage areas (reservoirs), channel, and sheet flow were estimated from proposed dimensions and parameters. The Modified Puls method was used to route flow through storage areas. The MODRAT method was used to route



flow through existing open channels and sub-basins. Proposed dimensions were used for all open channel routing. The discharge relationship from the storage areas used the Normal Depth method with similar dimensions. See Appendix 1 for a diagram of the entire watershed hydrologic model.

Hydraulic Model

Manning's Equation and Caltrans HDS No. 5 was used to simulate the hydraulic analysis of the existing and proposed storm drainage conveyance systems. The simulation of the hydraulic system utilized either the design storm event or the capacity of the existing system whichever was less. This capacity was defined from street grades, curb inlets, and assumed maximum energy gradients.

The main branch of Lyons Canyon will be diverted at debris basin #1. The remainder of downstream existing watershed is collected in a culvert near debris basin #2. A HEC-RAS model was developed for both existing and proposed conditions to show that adjacent properties are not adversely affected by the new culvert.

ASSUMPTIONS

The rainfall and runoff parameters are based on the County Hydrology Manual and the County Design Standards.

Rainfall

According to the isohyetal rainfall map in the County Hydrology Manual, the study area has an average 50-year, 24-hour rainfall depth of about 8.4 inches.

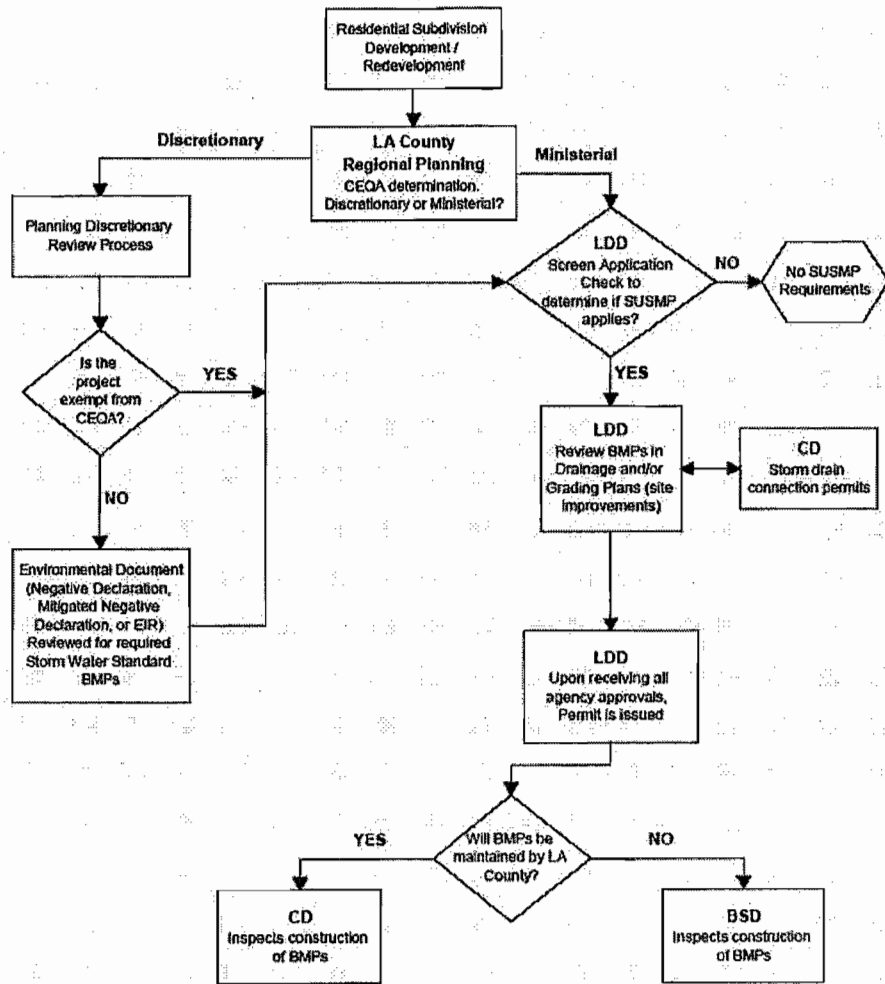
The mean annual precipitation is identified from DWR, Bulletin No. 195, October 1976. According to Plate 3, the MAP is about 18-inches. Plate 4 reports the mean annual 24-hour storm at roughly 3.5-inches.

SUSMP

The regulation governing the development of the project area is the County Standard Urban Storm Water Mitigation Plan (SUSMP). The project would fall under the subdivision development section. See Figure 5 for the permitting and inspection process for a residential development in the County. The subdivision improvements would need to be designed and installed according to this process.



Figure 5. SUSMP permitting & inspection process



DEFINITIONS
 BSD- LACDPW Building & Safety Division
 CD- LACDPW Construction Division
 LDD- LACDPW Land Development Division

Source control measures are recommended for implementation during specific project design, construction, and operations phases. These measures should include conserving natural areas, minimizing storm water pollutant of concern, protecting slopes and channels, providing storm drain stenciling and signage, properly design and construct outdoor material and refuse storage areas, and properly design and construct parking lots.

In addition to source control measures, treatment control Best Management Practices (BMPs) will need to be provided to remove the pollutants of concerns from the runoff



before leaving the site. Treatment control BMPs will require ongoing maintenance. BMPs can be provided in a variety of ways that can vary from catch basin filters, to proprietary treatment devices placed in the main storm drain infrastructure, to grass swale filters, to extended impoundment facilities that allow sedimentation of pollutants to occur. For large watersheds the use of catch basin filtration is not practical due to the number of installations necessary and the ongoing maintenance required. Debris basins (which could be considered extended detention basins or T-6 basins), debris inlets, catch basin filtration inserts, and centralized off-line devices (or water quality treatment devices) were selected as the initial BMPs. Due to the relatively long confinement period fencing (around each basin) is recommended to protect the facilities.

The other alternative BMPs identified above could be used in-lieu of the water quality devices and debris basins but they may not mitigate the increased peak runoff from the new development or help alleviate any adverse drainage conditions downstream of the project area.

See Table 3 for the proposed water quality devices. The SUSMP Manual specifies that 0.75 inches of rainfall be mitigated for water quality purposes. See Appendix 6 for the peak mitigated flow calculations.

Table 3. Water Quality Treatment Devices

Device #	node	Q _{pm} (cfs)
Off-line Device #1	94FJ	11.4
Off-line Device #2	99A+119S+123T	3.1
C.B. Insert #1	98A	1.0
C.B. Insert #2	128A	2.3
C.B. Insert #3	143Y	1.2

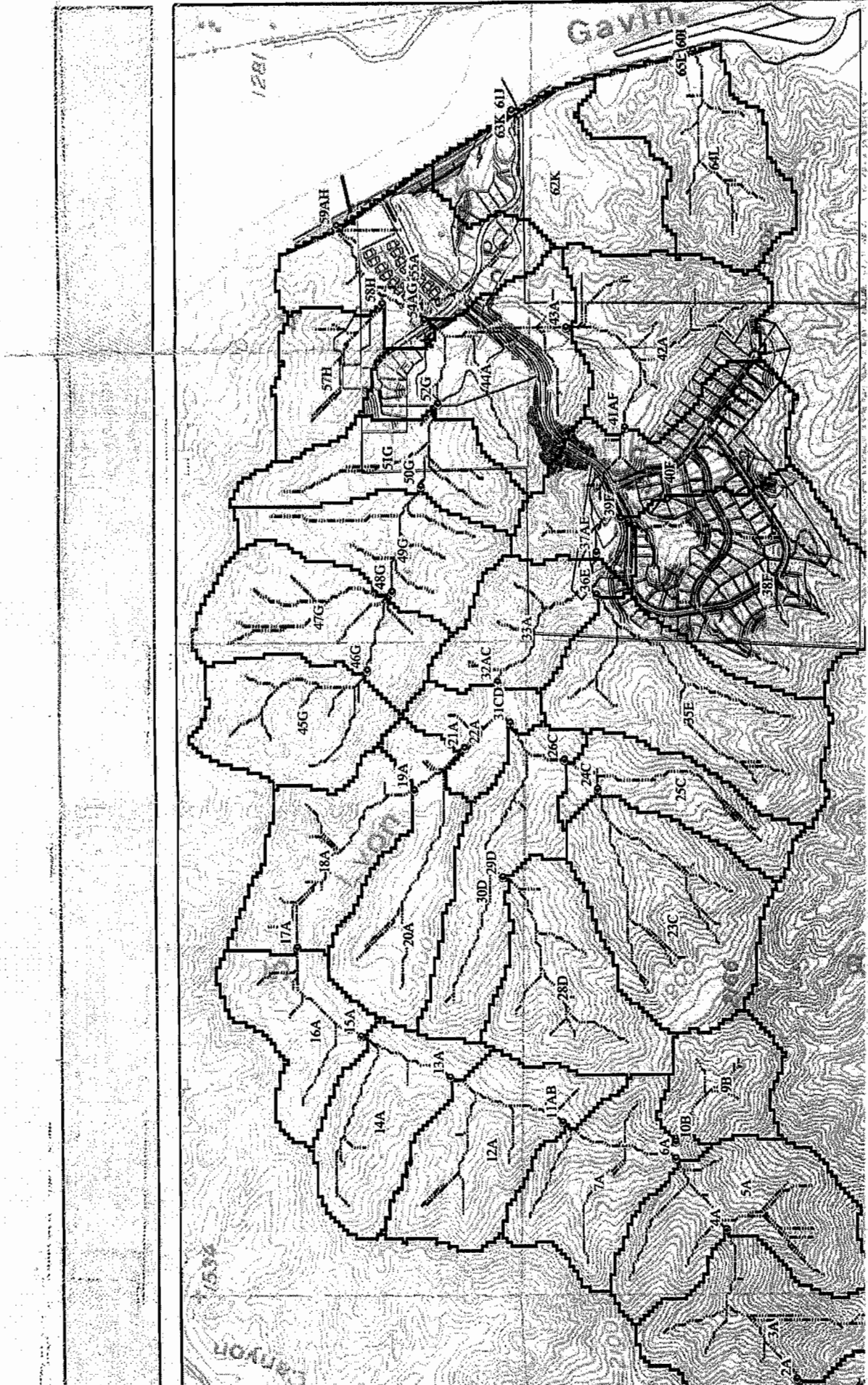


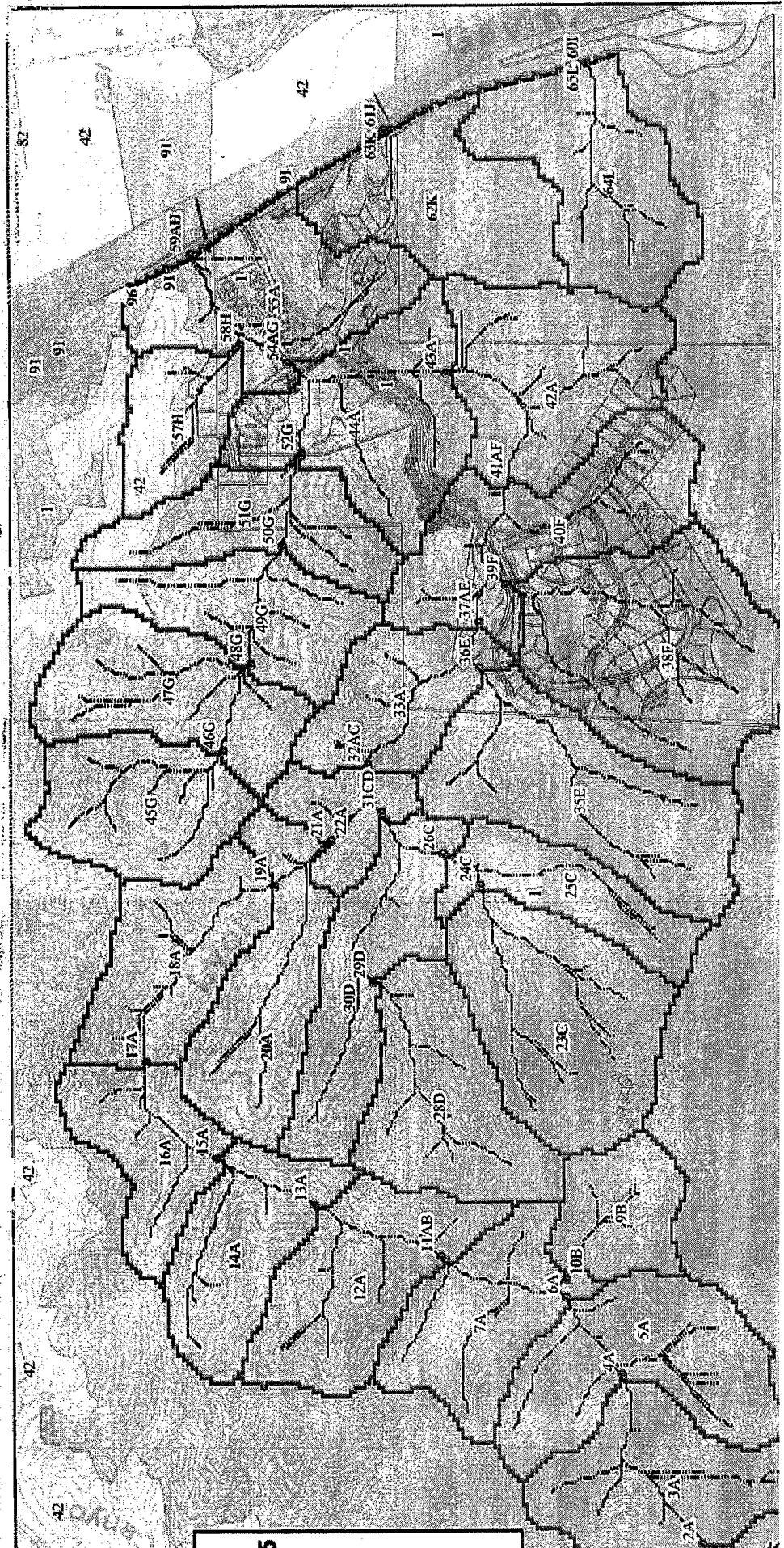
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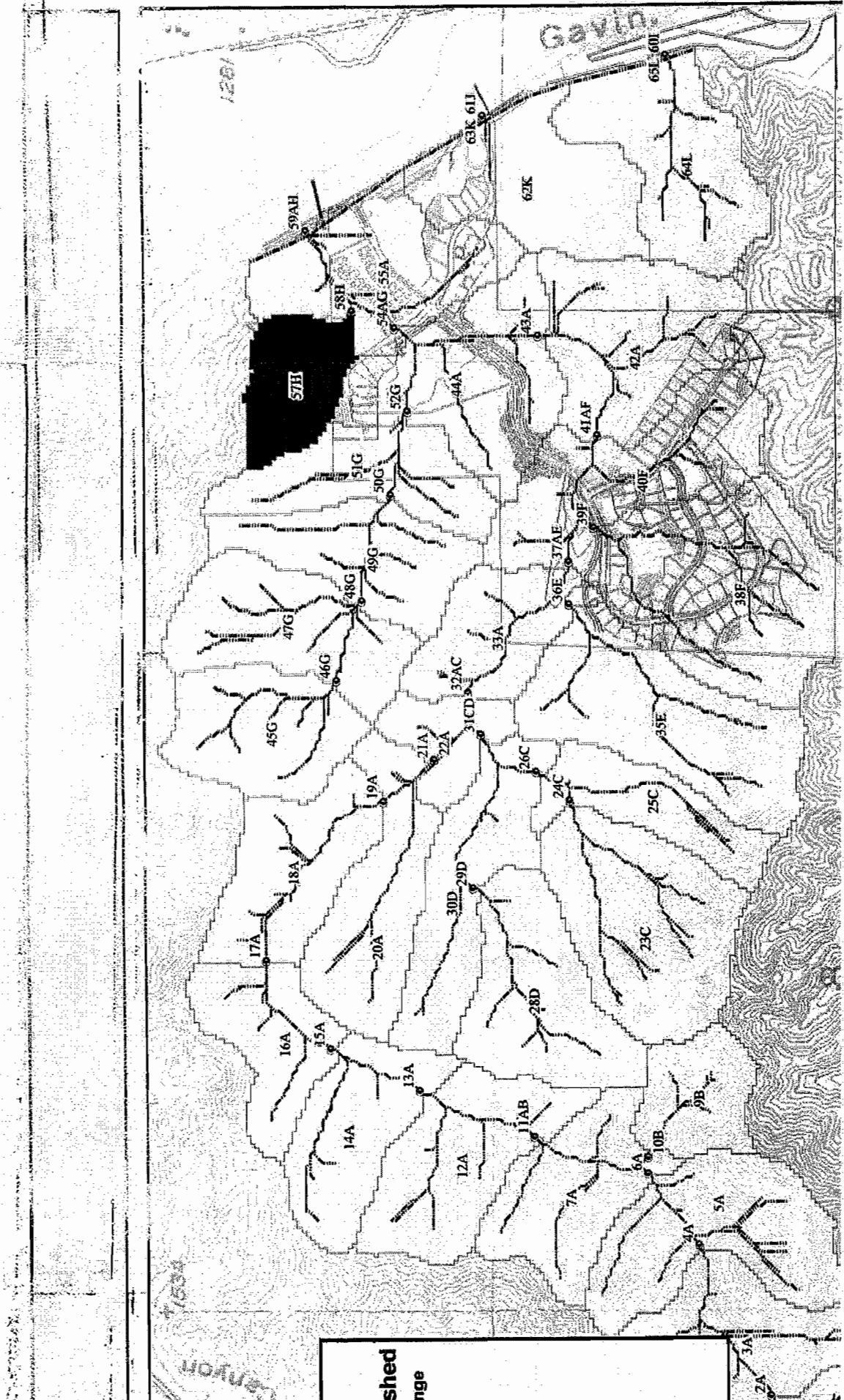
- 1) County of Los Angeles (2002). *Hydrology Manual for Department of Public Works*, Los Angeles, CA.
- 2) U.S. Soil Conservation Service (1985). *National Engineering Handbook, Section 4*. U.S. Department of Agriculture, Washington, D.C.
- 3) Pfrang, E.O., ed. (1992). "Design and Construction of Urban Stormwater Management Systems." *ASCE Manuals and Reports on Engineering Practice No. 77*, ASCE, New York NY.
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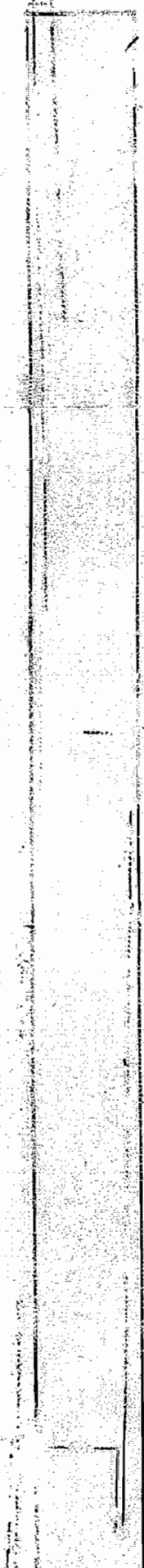


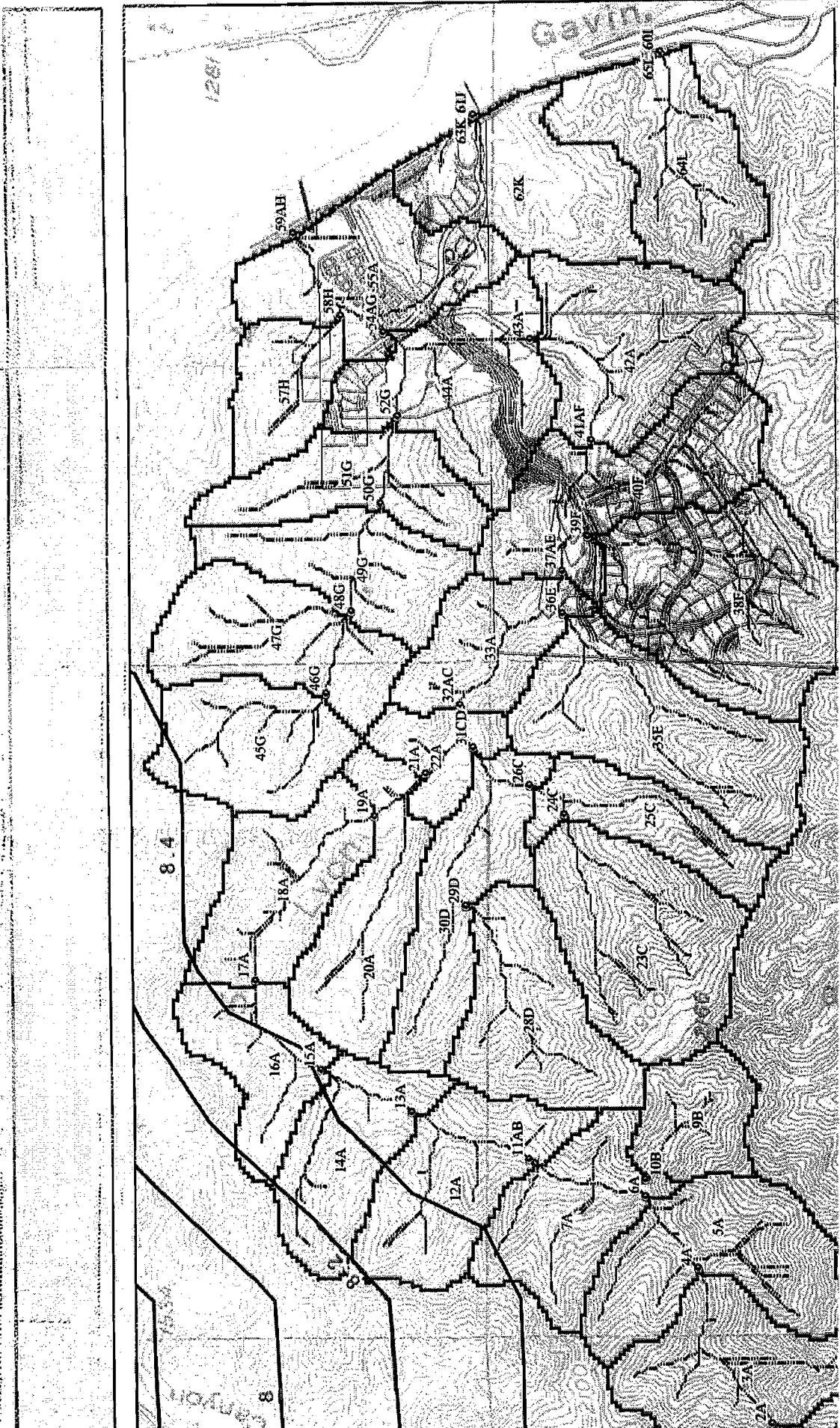


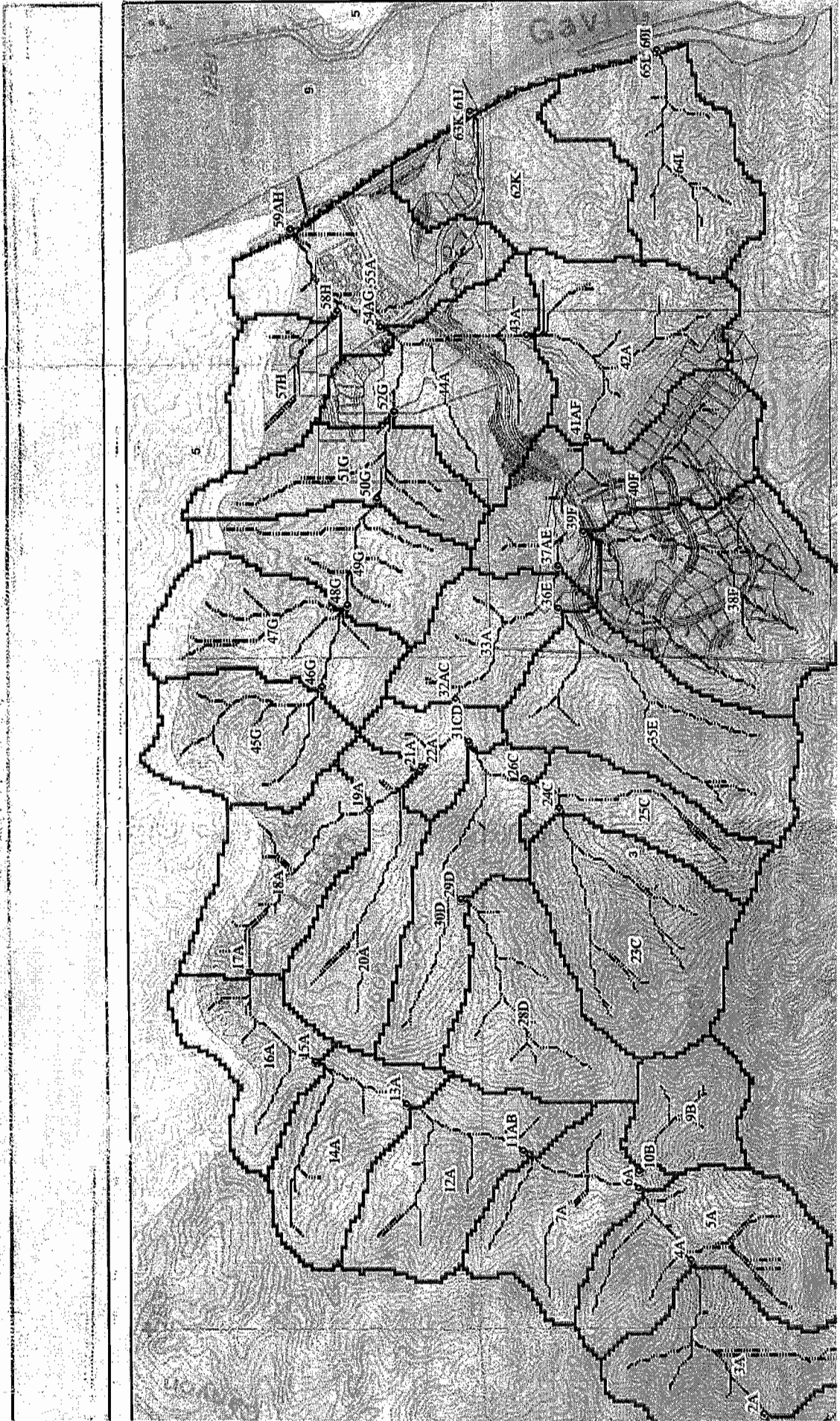


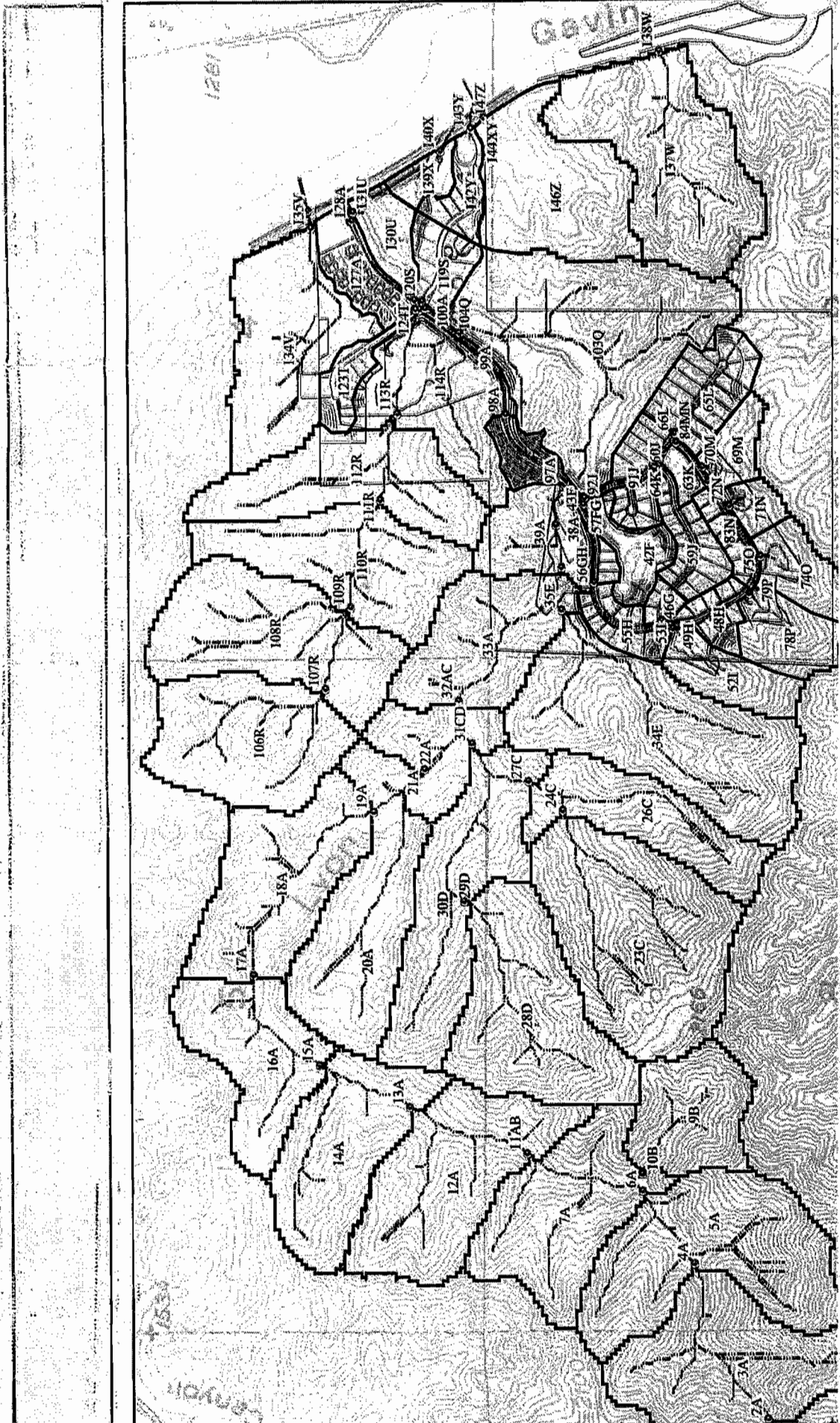
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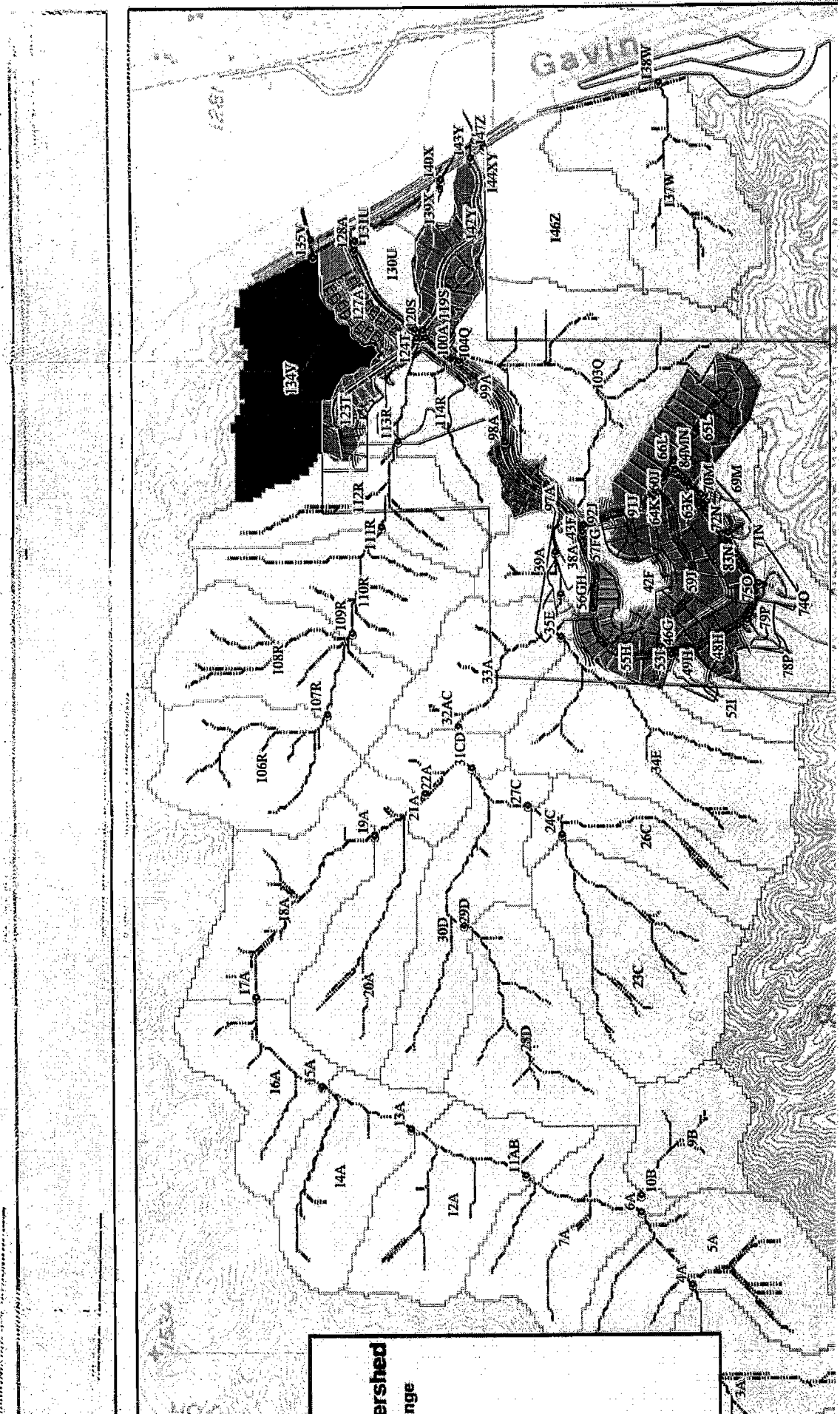
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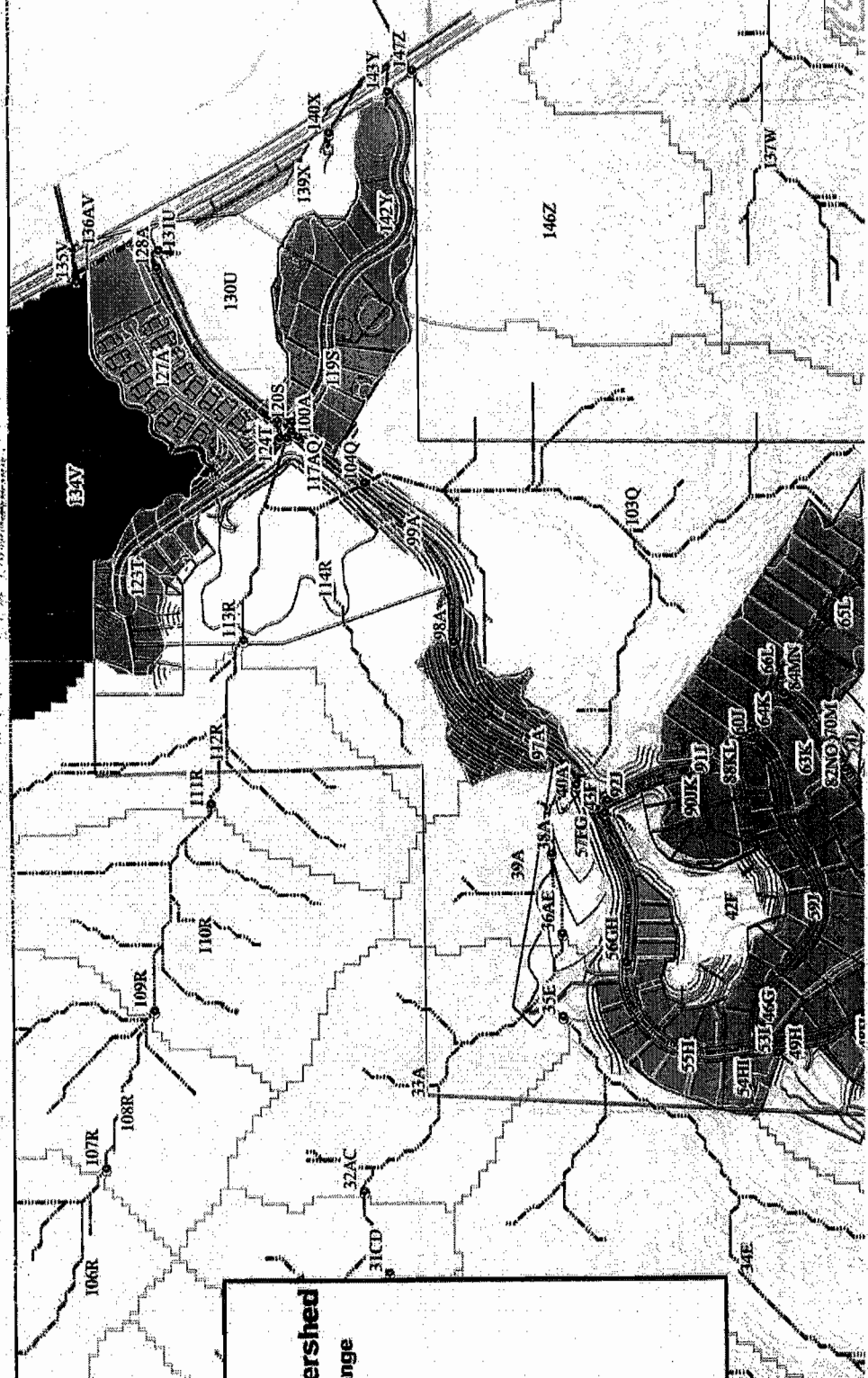




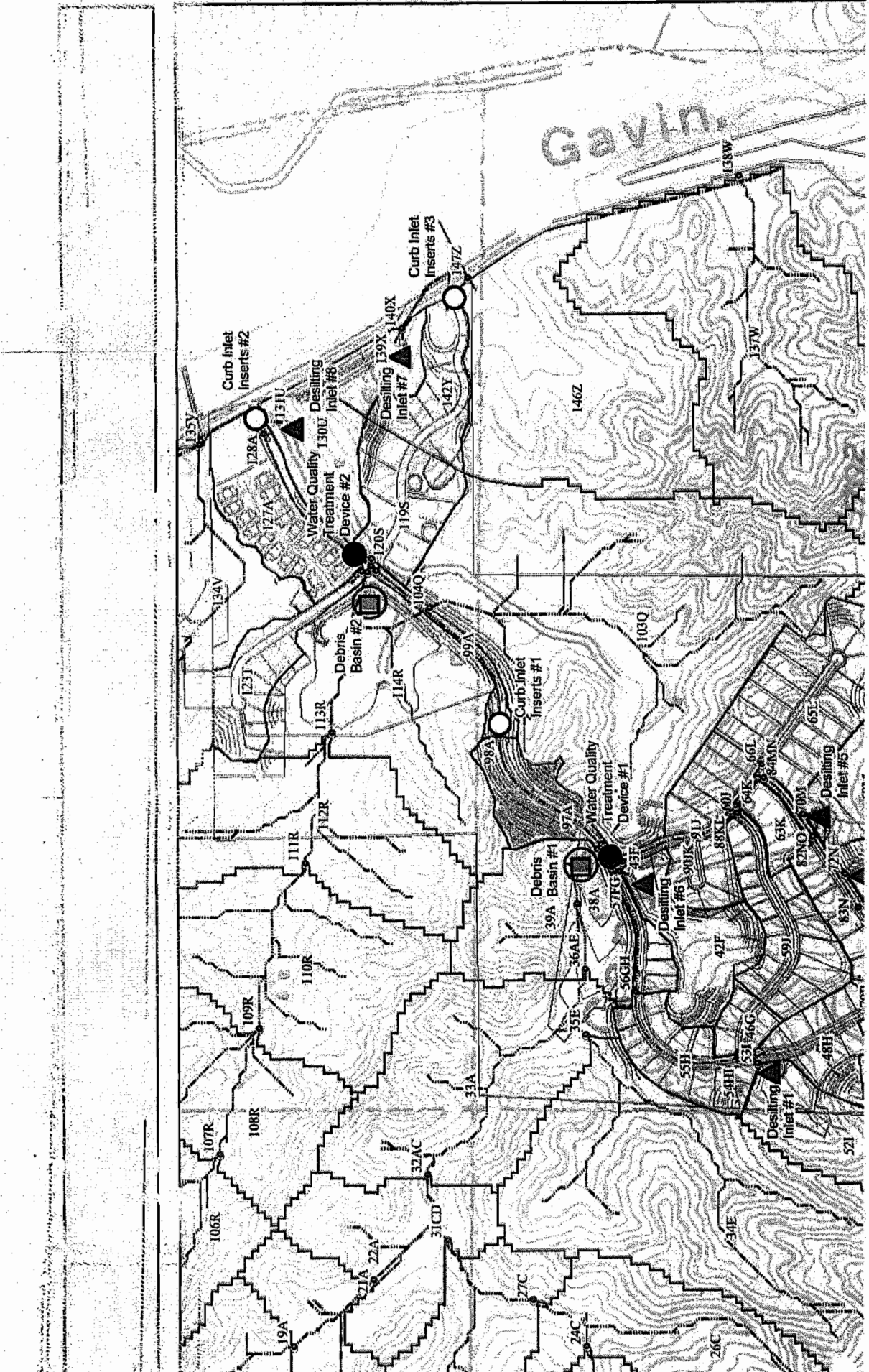


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Appendix 2
Existing Structures

Culvert Calculator Report 24 CSP

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	1,319.00 ft	Headwater Depth/Height	4.00
Computed Headwater Elev	1,319.00 ft	Discharge	19.53 cfs
Inlet Control HW Elev.	1,314.20 ft	Tailwater Elevation	1,311.00 ft
Outlet Control HW Elev.	1,319.00 ft	Control Type	Outlet Control

Grades			
Upstream Invert	1,311.00 ft	Downstream Invert	1,306.50 ft
Length	270.00 ft	Constructed Slope	0.016667 ft/ft

Hydraulic Profile			
Profile	Pressure Profile	Depth, Downstream	4.50 ft
Slope Type	N/A	Normal Depth	N/A ft
Flow Regime	N/A	Critical Depth	1.59 ft
Velocity Downstream	6.22 ft/s	Critical Slope	0.026963 ft/ft

Section			
Section Shape	Circular	Mannings Coefficient	0.024
Section Material	CMP	Span	2.00 ft
Section Size	24 Inch	Rise	2.00 ft
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	1,319.00 ft	Upstream Velocity Head	0.60 ft
Ke	0.90	Entrance Loss	0.54 ft

Inlet Control Properties			
Inlet Control HW Elev.	1,314.20 ft	Flow Control	N/A
Inlet Type	Projecting	Area Full	3.1 ft ²
K	0.03400	HDS # Chart	2
M	1.50000	HDS # Scale	3
C	0.05530	Equation Form	1
Y	0.54000		

Culvert Calculator Report

30 CSP

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	1,340.00 ft	Headwater Depth/Height	1.20
Computed Headwater Elev.	1,340.00 ft	Discharge	24.34 cfs
Inlet Control HW Elev.	1,339.89 ft	Tailwater Elevation	1,337.00 ft
Outlet Control HW Elev.	1,340.00 ft	Control Type	Outlet Control

Grades			
Upstream Invert	1,337.00 ft	Downstream Invert	1,336.30 ft
Length	65.00 ft	Constructed Slope	0.010769 ft/R

Hydraulic Profile			
Profile	M2	Depth, Downstream	1.68 ft
Slope Type	Mild	Normal Depth	2.21 ft
Flow Regime	Subcritical	Critical Depth	1.68 ft
Velocity Downstream	6.94 ft/s	Critical Slope	0.019080 ft/ft

Section			
Section Shape	Circular	Mannings Coefficient	0.024
Section Material	CMP	Span	2.50 ft
Section Size	30 inch	Rise	2.50 ft
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	1,340.00 ft	Upstream Velocity Head	0.48 ft
Ke	0.90	Entrance Loss	0.43 ft

Inlet Control Properties			
Inlet Control HW Elev.	1,339.89 ft	Flow Control	N/A
Inlet Type	Projecting	Area Fill	4.9 ft ²
K	0.03400	HDS 5 Chart	2
M	1.60000	HDS 5 Scale	3
C	0.06530	Equation Form	1
Y	0.54000		

Culvert Calculator Report

48 CSP

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	1,314.00 ft	Headwater Depth/Height	2.22
Computed Headwater Elev.	1,314.00 ft	Discharge	122.97 cfs
Inlet Control HW Elev.	1,312.54 ft	Tailwater Elevation	1,305.00 ft
Outlet Control HW Elev.	1,314.00 ft	Control Type	Outlet Control

Grades			
Upstream Invert	1,305.10 ft	Downstream Invert	1,304.00 ft
Length	140.00 ft	Constructed Slope	0.007857 ft/ft

Hydraulic Profile			
Profile	Composite M2 Pressure	Profile	
Slope Type	Mild	Depth, Downstream	3.33 ft
Flow Regime	Subcritical	Normal Depth	N/A ft
Velocity Downstream	10.99 ft/s	Critical Depth	3.33 ft
		Critical Slope	0.024273 ft/ft

Section			
Section Shape	Circular	Mannings Coefficient	0.024
Section Material	CMP	Span	4.00 ft
Section Size	48 Inch	Rise	4.00 ft
Number Sections	1		

Outlet Control Properties			
Outlet Control HW Elev.	1,314.00 ft	Upstream Velocity Head	1.49 ft
K _e	0.90	Entrance Loss	1.34 ft

Inlet Control Properties			
Inlet Control HW Elev.	1,312.54 ft	Flow Control	N/A
Inlet Type	Projecting	Area Full	12.6 ft ²
K	0.03400	HDS 5 Chart	2
M	1.50000	HDS 5 Scale	3
C	0.05530	Equation Form	1
Y	0.54000		

**Rating Table Report
2-8x8 RCB**

Range Data:

Minimum	Maximum	Increment	
Allowable HWE	1,296.00	1,310.00	1.00 ft

HW Elev. (ft)	Recharge (cfs)
1,296.00	0.00
1,297.00	40.94
1,298.00	116.81
1,299.00	212.76
1,300.00	327.56
1,301.00	457.78
1,302.00	601.76
1,303.00	758.31
1,304.00	928.48
1,305.00	1,105.51
1,306.00	1,294.79
1,307.00	1,421.27
1,308.00	1,560.88
1,309.00	1,706.44
1,310.00	1,838.66

Culvert Calculator Report

2-8x8 RCB

Solve For: Discharge

Culvert Summary			
Allowable HW Elevation	1,308.00 ft	Headwater Depth/Height	1.50
Computed Headwater Elev.	1,308.00 ft	Discharge	1,538.21 cfs
Inlet Control HW Elev.	1,308.00 ft	Tailwater Elevation	1,296.00 ft
Outlet Control HW Elev.	1,307.22 ft	Control Type	Inlet Control
Grades			
Upstream Invert	1,296.00 ft	Downstream Invert	1,293.00 ft
Length	300.00 ft	Constructed Slope	0.010000 ft/ft
Hydraulic Profile			
Profile	S2	Depth, Downstream	5.18 ft
Slope Type	Sleep	Normal Depth	4.95 ft
Flow Regime	Supercritical	Critical Depth	6.60 ft
Velocity Downstream	18.65 ft/s	Critical Slope	0.004814 ft/ft
Section			
Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	8.00 ft
Section Size	8 x 8 ft	Rise	8.00 ft
Number Sections	2		
Outlet Control Properties			
Outlet Control HW Elev.	1,307.22 ft	Upstream Velocity Head	3.30 ft
Ke	0.40	Entrance Loss	1.32 ft
Inlet Control Properties			
Inlet Control HW Elev.	1,308.00 ft	Flow Control	N/A
Inlet Type	30 to 75° wingwall flares	Area Full	128.0 ft²
K	0.02600	HDS 5 Chart	8
M	1.00000	HDS 5 Scale	1
C	0.03470	Equation Form	1
Y	0.86000		

4. SEDIMENT CONTROL

This Section discusses the type of structure acceptable to the Department for sediment control. The type of structure depends on the volume of sediment computed to be delivered to the site. This, in turn, depends on the Debris (sediment) Potential Area (DPA) zone for the particular watershed. The following table is used to determine the type of structure. See Section 3 for methods of computing the sediment production volume.

Total Sediment Production (cubic yards)	Type of Structure	
	DPA zone 1-4 requirement	DPA zone 5-11 requirement
20,000 or greater	Debris Basin	Debris Basin
5,000 to 19,999	Debris Basin	Elevated Inlet
1,000 to 4,999	Debris Basin or Elevated Inlet*	Desilting Inlet
250 to 999	Desilting Inlet*	Inlet with bulked flow drain
less than 250	Inlet with bulked flow drain	Inlet with bulked flow drain

* The use of elevated or desilting inlets and bulked flow drains in DPA zones 1 through 4 will only be approved by the Department in special circumstances. The reason being that the steepness of the watershed, presence of boulders, and higher sediment and runoff potential result in a greater risk of plugging the storm drain and damaging the desilting wall.

Table 4.1

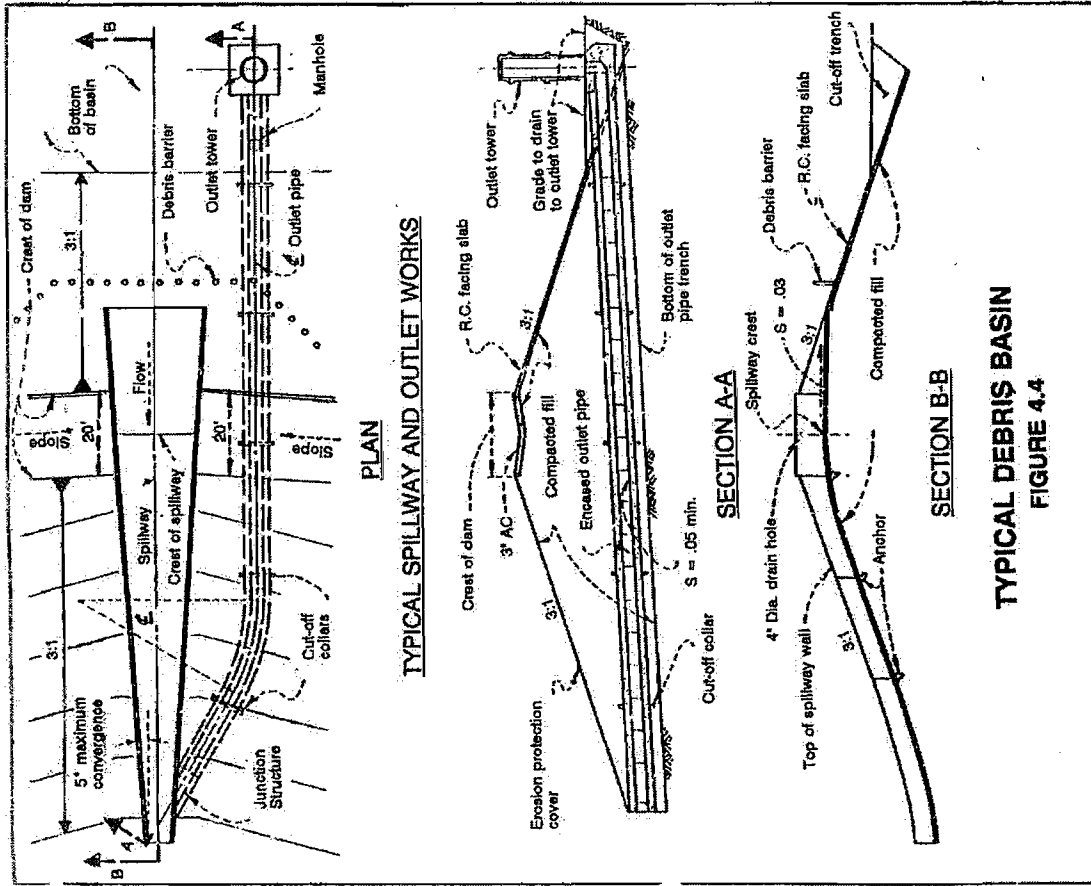
Where sediment production is less than 250 cubic yards, sediment control is generally not needed. Design the conveying storm drain following the closed conduit bulked flow design criteria listed in Section 5.D-2.

As stated in the State Water Code, Division 3, Section 6000-6452, certain dams are under State jurisdiction (refer to Figure 4.1). The State may have additional requirements for the design of the facility.

A. GENERAL DESIGN CONSIDERATIONS

A-1. Location and Alignment

Locate all sediment retaining facilities in the existing watercourse. Align dams perpendicular to the original flow paths (see Figure 4.2(a)). In order to insure maximum capacity, place the longer



TYPICAL SPILLWAY AND OUTLET WORKS

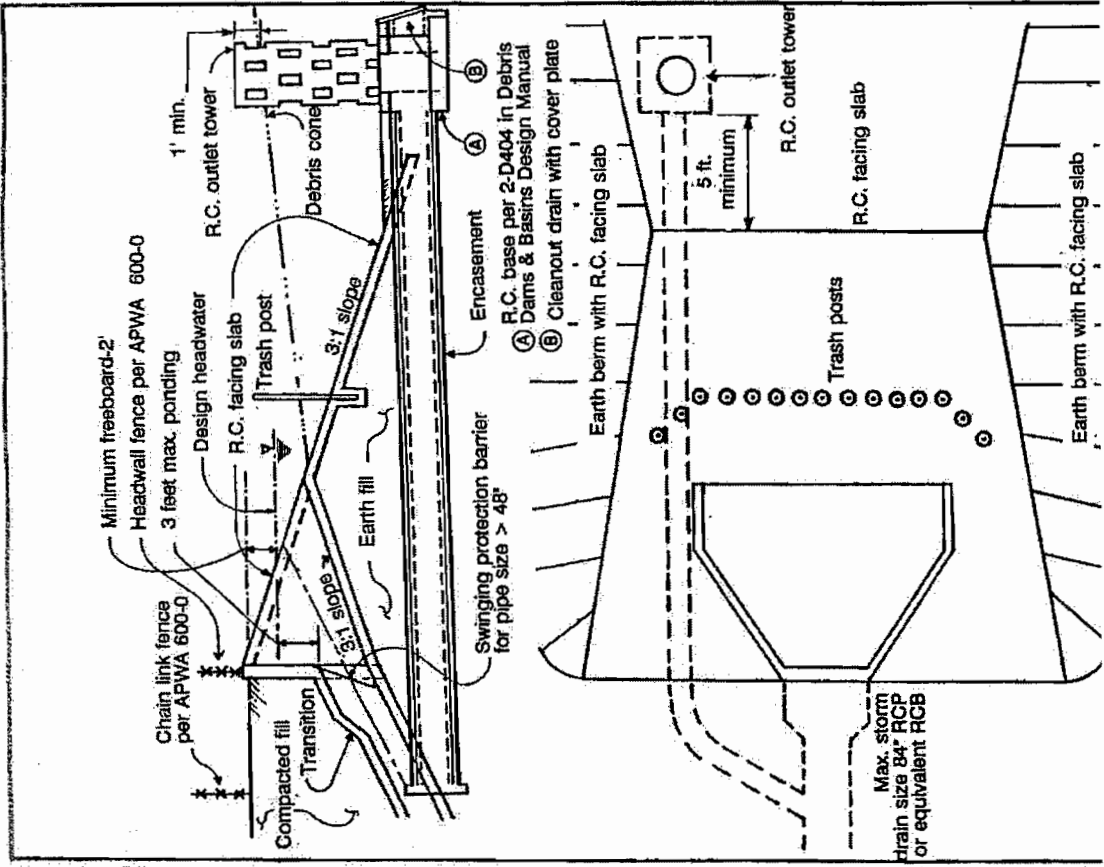
TYPICAL DEBRIS BASIN
FIGURE 4.4

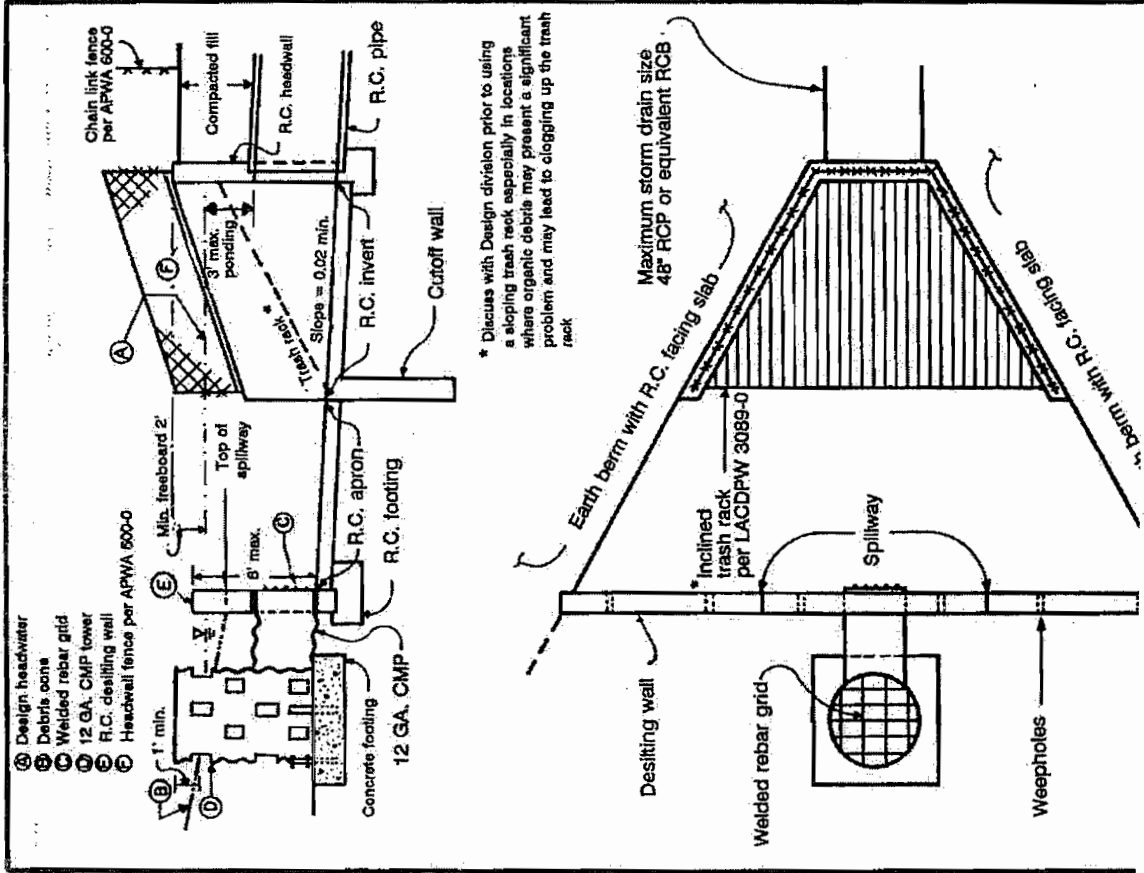
Table 4.2

	Debris Basin ¹	Elevated Inlet See Figure 4.5	
General Location		Locate both facilities such that should an overflow occur a street or other safe path is available to sediment.	
Horizontal alignment	Locate in the original watercourse where the dam is perpendicular to the flow path (see Figure 4.2(a)). Longer dimension of the basin shall fall along the flow.		
Outlet Tower and Conduit	Refer to the section on Outlet Works in the Department's Debris Dams and Basins Design Manual.	A standard concrete outlet tower and conduit is required (see the Debris Dams and Basin Design Manual), except in phased upstream development where corrugated metal pipe (CMP) tower with a concrete base may be substituted. ²	A corrugated metal pipe tower with a concrete base may be substituted. ²
Gage Boards	Gage boards are required on basins under State Jurisdiction. Sediment lines painted on towers, marking from the lowest port invert suffice for all others. See the section on Gage Board Pipe Support in the Department's Debris Dams and Basins Design Manual.	Gage boards or sediment lines painted on towers, marking from the lowest port invert can be used.	
Earth Embankment	Upstream and downstream embankment slopes less than or equal to 3H:1V. Steeper slopes require complete geotechnical stability analysis. Also refer to the section on Earthen Dam Design in the Department's Debris Dams and Basins Design Manual.	Maximum berm slope is 3H:1V. Steeper slopes require complete geotechnical stability analysis. Also refer to the section on Earth Dam Design in the Department's Debris Dams and Basins Design Manual.	Protect the desilting v concrete fa acceptable
Embankment Crest	The top width of the berm over the inlet shall be 20-feet paved 3 inches of asphalt concrete. A berm width of 15-feet may be approved if geological analysis is required.		
Facing Slab	6-inch concrete or gunite with No. 5 reinforcing steel at 18-inch spacing each way. See section on Earthen Dam Design, Protection for Dam Slopes in the Department's Debris Dams and Basins Design Manual.	A 6-inch thick reinforced concrete facing slab with reinforcing steel (no wire mesh) exterior placed concrete is acceptable. Provide facing slabs around the basin wall if cut and fill r	
Trash Barriers	Refer to the section on Debris Barrier in the Department's Debris Dams and Basins Design Manual.	A swinging trash rack is required for conduits greater than 48-inches in diameter. A sloping trash rack per LACDPW ³ 3089-0 can be used for smaller conduits. Trash posts spaced at 4-feet or 2/3 the diameter of the conduit, whichever is smaller, are also required at all elevated inlets.	A sloping trash post: conduit ar
Access Roads	Access roads with 12 ft wide paving (3-inch asphalt concrete on 4-inch crushed aggregate base) within a 15-ft easement with minimum radius of 40 feet can be used for structures with capacity less than 20,000 cubic yards. See section on Access to Dam and Basin in the Department's Debris Dams and Basins Design Manual.	Provide a vehicular access road into the basin at least 12-feet wide within a 15-foot easement with concrete over 4 inches of crushed aggregate base.	

Footnotes:

- ¹ Criteria listed in this table for debris basins amends the criteria given in the Department's Debris Dams and Basins Design Manual.
- ² The tower base can be modified to include a cleanout drain with a cover plate to allow flushing of the conduit. Extend the encasement on the conduit to the junction with from the intersection of the upstream face and the design headwater elevation meets the conduit, whichever is lesser.
- ³ Discuss with Design Division prior to using a sloping trash rack especially in locations where organic debris may present a significant problem and may lead to clogging up
- ⁴ Standard plans designated by an LACDPW number refer to the Department's Standard Plan Manual (1992 Edition).
- ⁵ Standard plans designated by an APWA number refer to the Standard Plans for Public Works Construction Manual by the American Public Works Association, 1985 Edition





- Ⓐ Design headwater
- Ⓑ Debris cone
- Ⓒ Weided rebar grid
- Ⓓ 12 GA. CMP tower
- Ⓔ R.C. desilting wall
- Ⓕ Headwall fence per APWA 600-0

* Discuss with Design division prior to using a sloping trash rack especially in locations where organic debris may present a significant problem and may lead to clogging up the trash rack

Maximum storm drain size 48" RCP or equivalent RCB

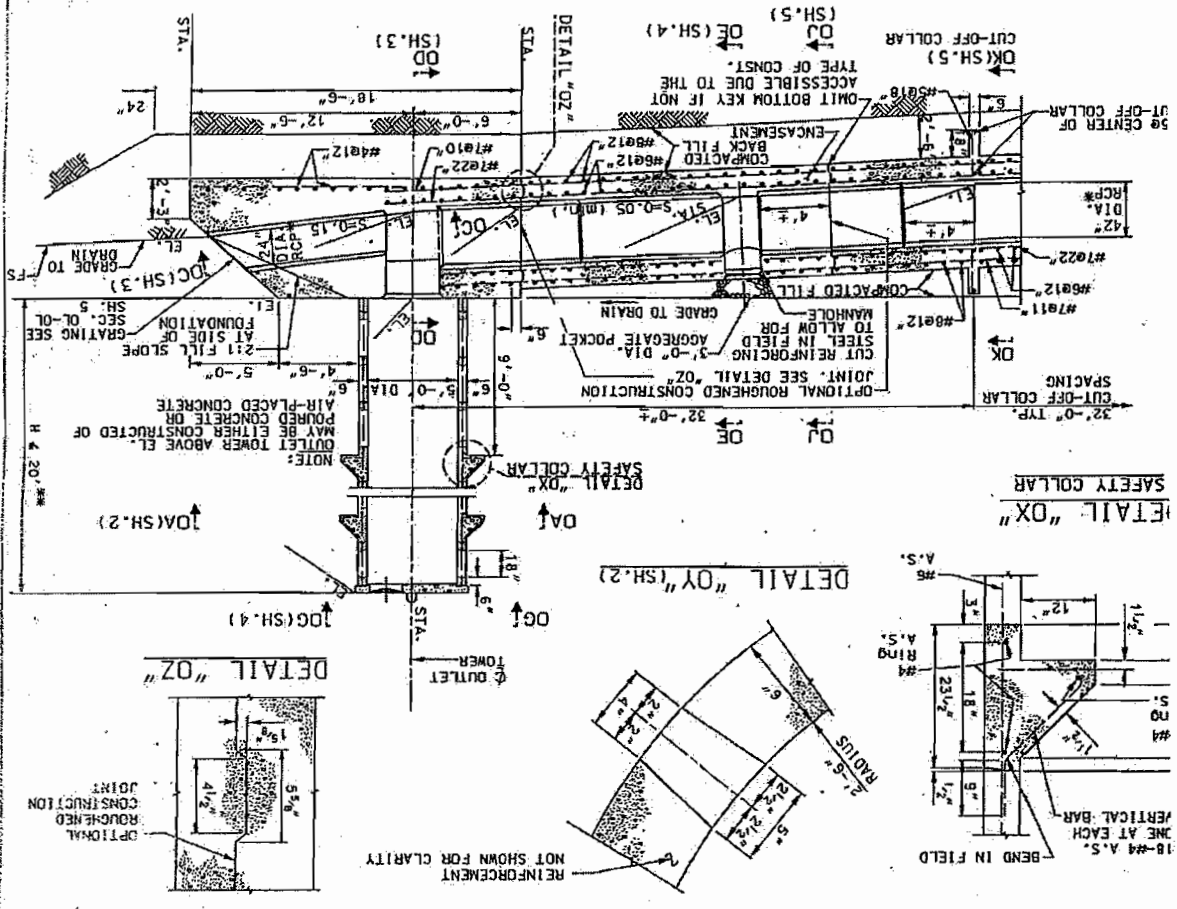
Weided rebar grid

Spillway

Weepholes

SECTION THROUGH OUTLET WORKS

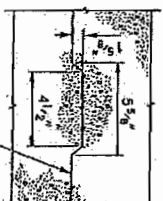
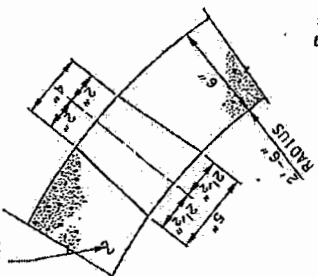
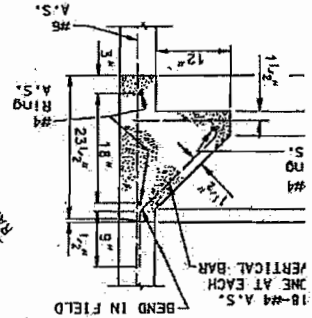
* - 2000 0 - MIN. INTERIOR STEEL COVER
 ** - FOR DESIGN H > 20' CONSULT
 PUBLIC WORKS DESIGN DIVISION

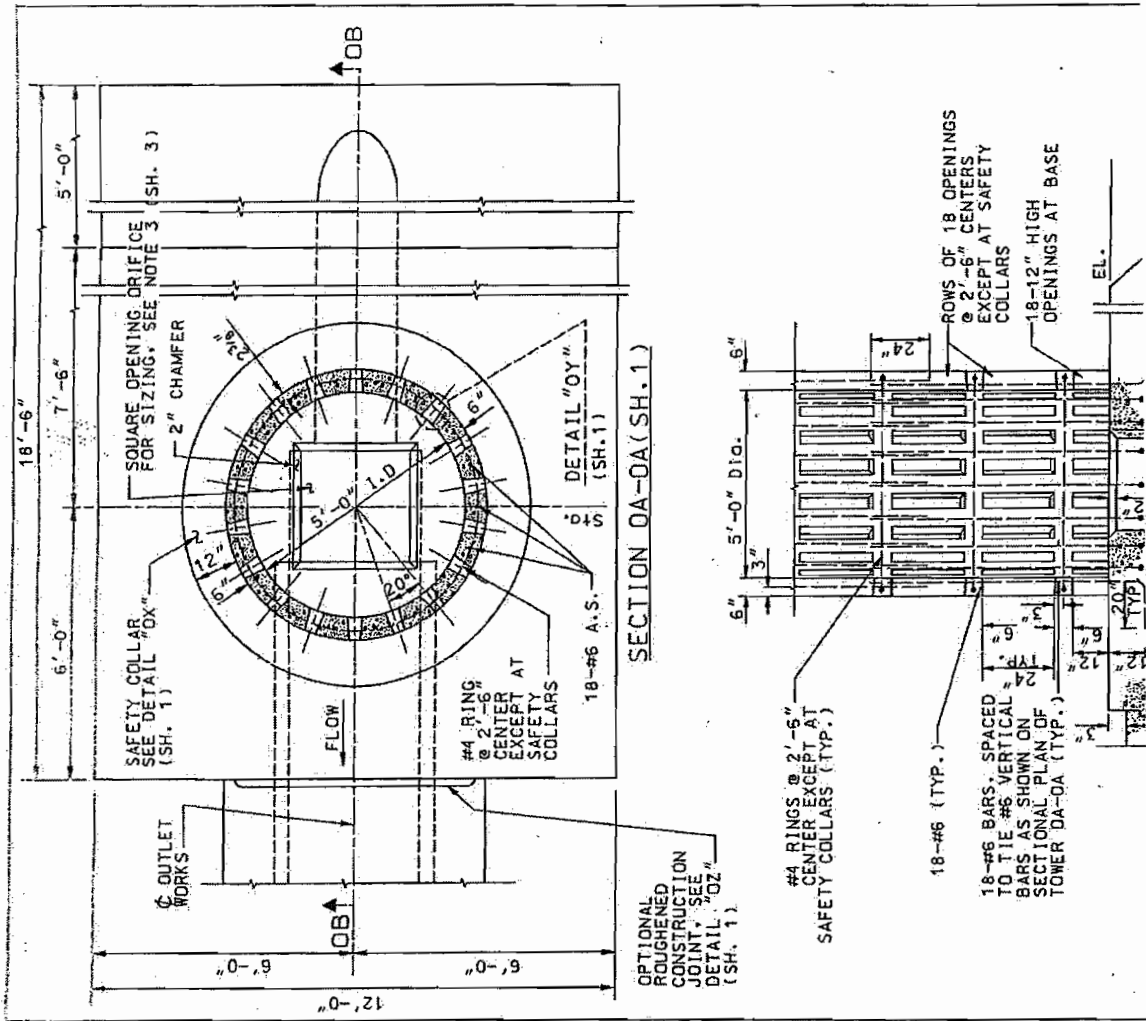


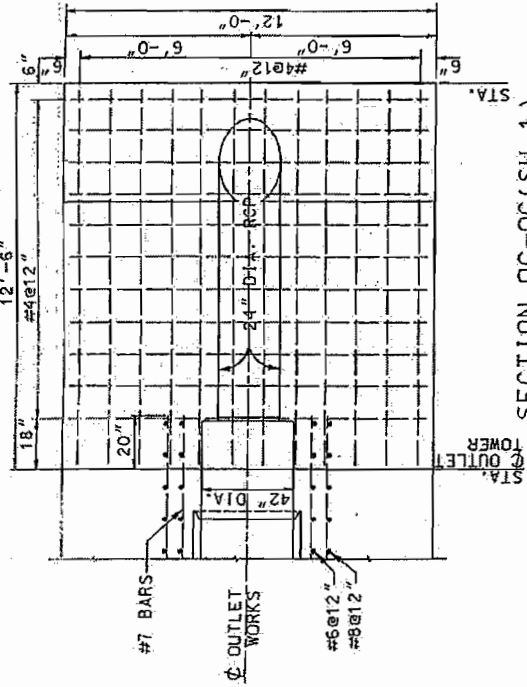
SAFETY COLLAR
 DETAIL "OX"

DETAIL "OY" (SH.2)

DETAIL "OZ"



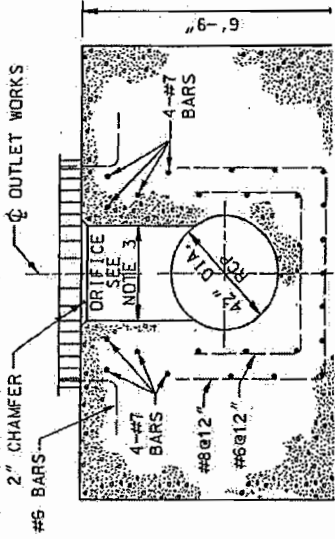




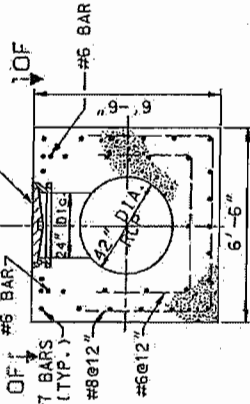
SECTION OC-OC(SH.1)

NOTE:

1. THE 42" RCP SHALL BE SECURELY ANCHORED WHILE CONCRETE ENCASMENT IS BEING PLACED.
2. THE LONGITUDINAL #7 BARS SHOWN IN THE OUTLET PIPE ENCASMENT ARE DESIGNED FOR HARD GRADE STEEL ONLY. IF MEDIUM GRADE STEEL IS USED IN PLACE OF HARD GRADE STEEL, THE SIZE OR NUMBER OF LONGITUDINAL BARS MUST BE INCREASED TO PROVIDE A TOTAL STEEL AREA EQUAL TO 1.25 TIMES THE TOTAL AREA OF LONGITUDINAL BARS SHOWN HEREON.
3. THE ORIFICE OPENING SIZE SHALL BE SIZED FOR NON-PRESSURE FLOW AT MAXIMUM HEAD WITH A MINIMUM DISCHARGE OF 150 CFS

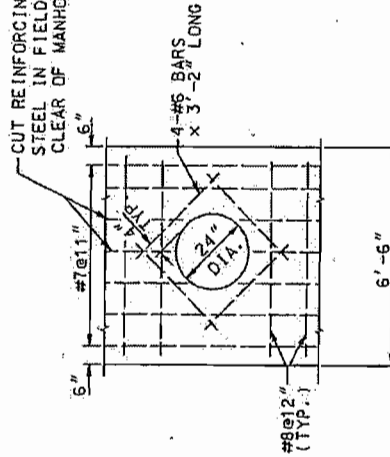


MANHOLE FRAME AND COVER AS PER STD. PLAN 530-1. DRILLED $\frac{5}{8}$ " DIA. HOLES THROUGH COVER IN A UNIFORM PATTERN. NO HOLES ARE TO BE DRILLED THROUGH THE RIBS OF THE COVER. BOTTOM MAKE TOP OF COVER FLUSH WITH TOP OF ENCASEMENT.

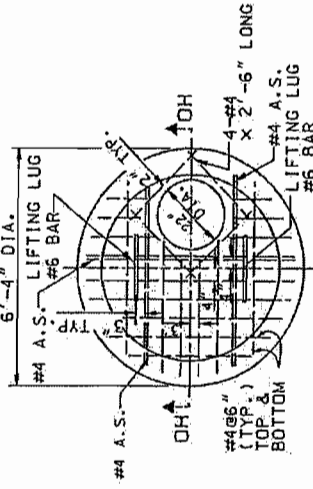


SECTION OE-OE (SH.1)

CUT REINFORCING STEEL IN FIELD 2" CLEAR OF MANHOLE

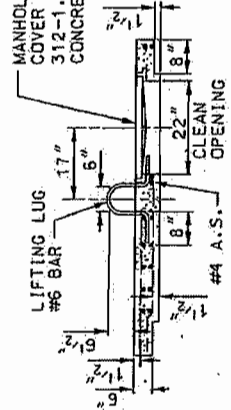


SECTION OF -OF

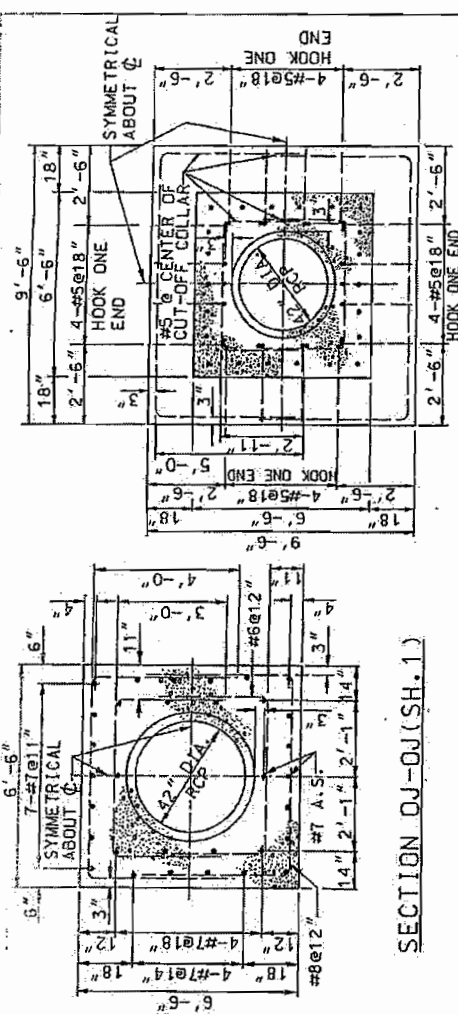


PLAN OG-DG (SH.1)
SHOWING OUTLET TOWER COVER

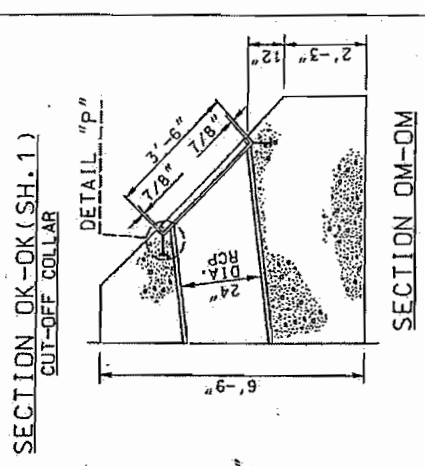
MANHOLE FRAME AND COVER PER STD. PLAN 312-1, SET FLUSH WITH CONCRETE SURFACE



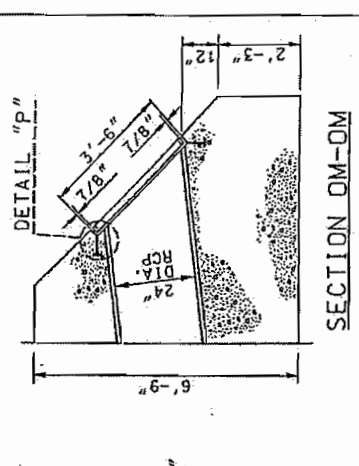
SECTION OF -OF



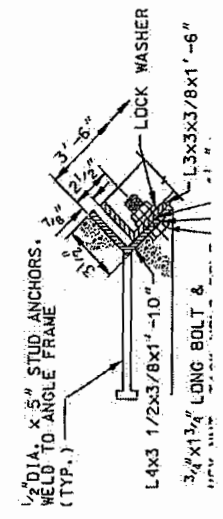
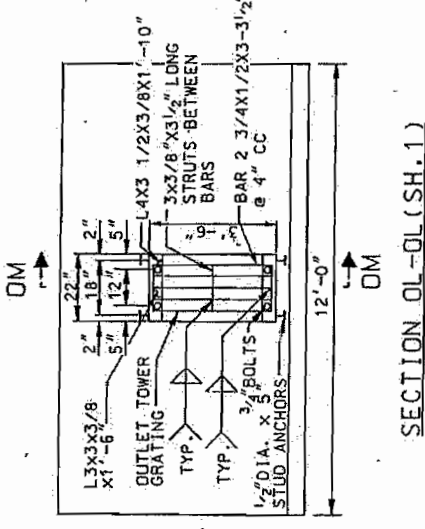
SECTION OJ-OJ (SH.1)



SECTION OK-OK (SH.1)



SECTION OL-OL (SH.1)



1/2" DIA. x 5" STUD ANCHORS WELDED TO ANGLE FRAME (TYP.)

LOCK WASHER

L3x3x3/8x1'-6" LONG BOLT &

Appendix 4
Existing Burned Watershed
50-Years, 24-hour event

Appendix 4
Existing Burned Watershed
50-Years, 24-hour event

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 LIAR STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows range from 500 to 1400 minutes.

Total Runoff = 40.714 Acre-Ft.
Peak Q = 473.08 CFS
Time to Peak = 1162 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 LIAR STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows range from 500 to 1400 minutes.

Total Runoff = 40.783 Acre-ft.
Peak Q = 462.71 CFS
Time to Peak = 1164 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 32AC STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows range from 500 to 1145 minutes.

Why are these hydrographs included?

Total Runoff = 28.249 Acre-ft
Peak Q = 283.66 CFS
Time to Peak = 1166 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 52RY STORM DAY 4 STORM FREQ. 56 REDUCTION FACTOR = 1.000

Table with columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows show discharge values from 0 to 1400 at 100-minute intervals.

Total Runoff = 28.291 Acre-ft.
Peak Q = 273.70 CFS
Time to Peak = 1166 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 54AG STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows show discharge values from 0 to 1400 at 100-minute intervals.

Total Runoff = 238.111 Acre-ft.
Peak Q = 1310.17 CFS
Time to Peak = 1171 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 54RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows show discharge values at 0, 500, and 1000 minutes.

1290	1.85	1291	1.83	1292	1.83	1293	1.81	1294	1.80
1295	1.83	1296	1.79	1297	1.77	1298	1.77	1299	1.76
1300	1.77	1301	1.64	1302	1.58	1303	1.52	1304	1.46
1305	1.47	1306	1.37	1307	1.30	1308	1.27	1309	1.23
1310	1.23	1311	1.12	1312	1.07	1313	0.99	1314	0.92

Total Runoff = 8.611 Acre-ft.
 Peak Q = 112.51 CFS
 Time to Peak = 1158 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
 TOTAL HYDROGRAPH AT EBL STORM DRY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

TIME	Q	TIME	Q	TIME	Q	TIME	Q	TIME	Q
0	0.00	100	9.98	200	1.08	300	1.15	400	1.25
500	1.40	600	1.52	700	1.65	800	2.29	900	3.52
1000	6.38	1050	8.09	1100	14.37	1110	16.26	1120	18.93
1130	22.98	1131	23.44	1132	23.96	1133	24.49	1134	25.07
1135	25.71	1136	26.37	1137	27.06	1138	27.83	1139	28.65
1140	29.56	1141	30.54	1142	31.62	1143	32.80	1144	34.10
1145	35.58	1146	37.24	1147	38.95	1148	40.87	1149	43.07
1150	45.68	1151	48.82	1152	52.74	1153	57.50	1154	63.85
1155	73.61	1156	103.73	1157	115.71	1158	116.68	1159	115.21
1160	112.12	1161	107.55	1162	101.16	1163	92.43	1164	80.09
1165	47.62	1166	32.53	1167	27.69	1168	24.64	1169	22.46
1170	20.61	1171	19.06	1172	17.79	1173	16.72	1174	15.82
1175	15.03	1176	14.33	1177	13.60	1178	12.94	1179	12.36
1180	11.84	1181	11.35	1182	10.92	1183	10.53	1184	10.16
1185	9.83	1186	9.52	1187	9.23	1188	8.96	1189	8.71
1190	8.47	1191	8.26	1192	8.04	1193	7.86	1194	7.68
1195	7.49	1196	7.34	1197	7.14	1198	6.92	1199	6.71
1200	6.51	1201	6.32	1202	6.13	1203	5.97	1204	5.82
1205	5.66	1206	5.51	1207	5.35	1208	5.23	1209	5.09
1210	4.97	1211	4.86	1212	4.72	1213	4.63	1214	4.51
1215	4.41	1216	4.32	1217	4.22	1218	4.13	1219	4.04
1220	3.95	1221	3.87	1222	3.78	1223	3.72	1224	3.63
1225	3.58	1226	3.49	1227	3.43	1228	3.38	1229	3.31
1230	3.25	1231	3.18	1232	3.13	1233	3.07	1234	3.01
1235	2.97	1236	2.91	1237	2.86	1238	2.81	1239	2.76
1240	2.72	1241	2.67	1242	2.63	1243	2.59	1244	2.54
1245	2.49	1246	2.46	1247	2.42	1248	2.38	1249	2.35
1250	2.31	1251	2.28	1252	2.24	1253	2.21	1254	2.18
1255	2.16	1256	2.15	1257	2.13	1258	2.11	1259	2.09
1260	2.08	1261	2.06	1262	2.04	1263	2.03	1264	2.01
1265	2.00	1266	1.99	1267	1.97	1268	1.96	1269	1.94
1270	1.93	1271	1.91	1272	1.90	1273	1.89	1274	1.87
1275	1.86	1276	1.85	1277	1.83	1278	1.82	1279	1.81
1280	1.80	1281	1.79	1282	1.77	1283	1.76	1284	1.75
1285	1.75	1286	1.73	1287	1.72	1288	1.71	1289	1.70
1290	1.69	1291	1.68	1292	1.67	1293	1.66	1294	1.65
1295	1.64	1296	1.63	1297	1.62	1298	1.61	1299	1.60
1300	1.60	1310	1.51	1320	1.44	1330	1.37	1340	1.32
1350	1.26	1360	1.21	1370	1.17	1380	1.13	1390	1.09
1400	1.06	1420	1.00	1440	0.94	1460	0.00	1500	0.00

Total Runoff = 8.501 Acre-ft.
 Peak Q = 116.68 CFS
 Time to Peak = 1158 Minutes

LOS ANGELES COUNTY DEPARTMENT OF PUBLIC WORKS
MODIFIED RATIONAL METHOD HYDROLOGY
RESERVOIR ROUTING OUTPUT

***** RESERVOIR ROUTING STORM DAY 4 *****

RESERVOIR ROUTING at 59AH STORM DAY 4 STORM FREQ. 50
INITIAL WATER SURFACE ELEVATION: 1296.00
RESERVOIR COMPOSITE ELEVATION-STORAGE-DISCHARGE DATA at 59AH

ELEVATION (ft.)	STORAGE (a.f.)	OUTFLOW (cfs)
1296.00	0.00	0.00
1297.00	0.00	40.90
1298.00	0.00	115.80
1299.00	0.00	212.80
1300.00	0.16	327.60
1301.00	0.33	457.80
1302.00	0.49	601.80
1303.00	0.65	758.30
1304.00	0.82	926.50
1305.00	0.98	1105.50
1306.00	1.15	1294.80
1307.00	1.31	1414.60
1308.00	1.47	1538.20
1309.00	1.64	1670.80
1310.00	1.80	1793.70

RESERVOIR DISCHARGE DATA: 59AH Known discharge 1

ELEVATION (ft.)	OUTFLOW (cfs)
1296.00	0.00
1297.00	40.90
1298.00	115.80
1299.00	212.80
1300.00	327.60
1301.00	457.80
1302.00	601.80
1303.00	758.30
1304.00	926.50
1305.00	1105.50
1306.00	1294.80
1307.00	1414.60
1308.00	1538.20
1309.00	1670.80
1310.00	1793.70

*how was
this discharge
table obtained?*

RESERVOIR ROUTING TABLE at 59AH

TIME	INFLOW (cfs)	OUTFLOW (cfs)	W.S.ELEV (ft.)	STORAGE (a.f.)
0	3.20	0.00	1296.00	0.00
100	13.50	10.30	1296.25	0.00
200	30.72	27.52	1296.67	0.00
300	37.50	34.30	1296.84	0.00
400	43.05	39.85	1296.97	0.00
500	51.00	47.80	1297.09	0.00
600	59.24	56.04	1297.20	0.00
700	69.41	66.21	1297.34	0.00
800	82.77	79.57	1297.52	0.00
900	107.93	104.73	1297.85	0.00
1000	161.46	158.26	1298.44	0.00
1050	208.55	205.35	1298.92	0.00
1100	301.07	299.20	1299.75	0.12
1110	327.72	325.02	1299.98	0.16
1120	360.80	357.66	1300.23	0.20
1130	406.98	402.60	1300.58	0.26

1131	412.34	407.47	1300.61	0.26
1132	417.75	412.70	1300.65	0.27
1133	423.27	418.09	1300.70	0.28
1134	429.02	423.65	1300.74	0.29
1135	435.05	429.44	1300.78	0.29
1136	441.33	435.49	1300.83	0.30
1137	447.91	441.80	1300.88	0.31
1138	454.78	448.39	1300.93	0.32
1139	461.96	455.28	1300.98	0.33
1140	469.53	463.02	1301.04	0.34
1141	477.49	471.05	1301.09	0.34
1142	485.92	479.20	1301.15	0.35
1143	494.81	487.75	1301.21	0.36
1144	504.28	496.78	1301.27	0.37
1145	514.35	506.37	1301.34	0.38
1146	525.09	516.59	1301.41	0.40
1147	536.66	527.52	1301.48	0.41
1148	548.95	539.22	1301.57	0.42
1149	562.25	551.75	1301.65	0.43
1150	576.75	565.33	1301.75	0.45
1151	592.66	580.16	1301.85	0.47
1152	610.31	596.48	1301.96	0.48
1153	630.26	615.37	1302.09	0.50
1154	652.94	636.49	1302.22	0.53
1155	680.53	660.84	1302.38	0.55
1156	726.87	695.34	1302.60	0.59
1157	763.15	735.32	1302.85	0.63
1158	798.93	772.22	1303.08	0.66
1159	842.87	811.68	1303.32	0.70
1160	892.07	856.90	1303.59	0.75
1161	946.11	907.31	1303.89	0.80
1162	1002.32	964.12	1304.21	0.85
1163	1058.10	1021.65	1304.53	0.91
1164	1111.75	1076.72	1304.84	0.95
1165	1161.70	1128.88	1305.12	1.00
1166	1205.83	1176.53	1305.38	1.04
1167	1227.67	1211.45	1305.56	1.08
1168	1252.45	1236.28	1305.69	1.10
1169	1274.75	1259.99	1305.82	1.12
1170	1292.99	1280.72	1305.93	1.14
1171	1308.70	1297.46	1306.02	1.15
1172	1321.98	1309.63	1306.12	1.17
1173	1331.64	1321.32	1306.22	1.19
1174	1336.19	1329.89	1306.29	1.20
1175	1334.71	1333.67	1306.32	1.20
1176	1326.97	1331.75	1306.31	1.20
1177	1313.33	1323.86	1306.24	1.19
1178	1294.52	1310.30	1306.13	1.17
1179	1271.72	1290.98	1305.98	1.15
1180	1245.87	1263.04	1305.83	1.12
1181	1217.69	1235.90	1305.69	1.10
1182	1187.84	1207.14	1305.54	1.07
1183	1156.79	1176.91	1305.38	1.04
1184	1125.46	1145.84	1305.21	1.02
1185	1093.99	1114.49	1305.05	0.99
1186	1063.02	1083.19	1304.88	0.96
1187	1032.45	1052.33	1304.70	0.93
1188	1002.45	1021.97	1304.53	0.91
1189	972.90	992.12	1304.37	0.88
1190	943.72	962.69	1304.20	0.85
1191	914.71	933.55	1304.04	0.83
1192	885.95	906.14	1303.88	0.80
1193	857.15	878.10	1303.71	0.77
1194	828.68	849.58	1303.54	0.74

1194	800.37	821.17	1303.37	0.71
1195	792.47	791.96	1303.21	0.69
1197	744.97	761.19	1303.04	0.66
1198	717.90	711.97	1302.87	0.63
1199	691.42	711.16	1302.70	0.60
1200	665.73	684.93	1302.53	0.57
1201	640.85	659.46	1302.37	0.55
1202	616.84	634.82	1302.21	0.52
1203	593.73	611.04	1302.06	0.50
1204	571.49	588.83	1301.91	0.48
1205	550.22	567.42	1301.76	0.45
1206	529.86	546.47	1301.62	0.43
1207	510.42	526.32	1301.48	0.41
1208	491.90	507.06	1301.34	0.38
1209	474.24	488.70	1301.21	0.36
1210	457.44	471.20	1301.09	0.34
1211	441.48	454.88	1300.98	0.33
1212	426.32	440.39	1300.87	0.31
1213	411.95	425.71	1300.75	0.29
1214	398.45	411.55	1300.64	0.27
1215	385.66	398.08	1300.54	0.25
1216	373.37	385.26	1300.44	0.24
1217	361.59	372.98	1300.35	0.22
1218	350.31	361.22	1300.26	0.20
1219	339.85	350.07	1300.17	0.19
1220	329.82	339.55	1300.09	0.18
1221	320.20	329.51	1300.01	0.16
1222	311.26	320.31	1299.94	0.15
1223	302.76	311.51	1299.86	0.14
1224	294.57	303.01	1299.79	0.13
1225	287.15	294.98	1299.72	0.11
1226	280.02	287.44	1299.65	0.10
1227	273.14	280.26	1299.59	0.09
1228	266.48	273.35	1299.53	0.08
1229	260.04	266.67	1299.47	0.08
1230	253.83	260.23	1299.41	0.07
1231	247.82	254.01	1299.36	0.06
1232	242.04	248.00	1299.31	0.05
1233	236.44	242.21	1299.26	0.04
1234	231.03	236.61	1299.21	0.03
1235	225.84	231.20	1299.16	0.03
1236	220.81	225.99	1299.11	0.02
1237	215.98	220.97	1299.07	0.01
1238	211.30	216.12	1299.03	0.00
1239	206.82	208.72	1298.96	0.00
1240	202.67	200.77	1298.88	0.00
1241	198.97	200.87	1298.88	0.00
1242	195.37	193.47	1298.80	0.00
1243	191.86	193.76	1298.80	0.00
1244	188.45	186.55	1298.73	0.00
1245	185.14	187.04	1298.73	0.00
1246	181.91	180.01	1298.66	0.00
1247	178.75	180.65	1298.67	0.00
1248	175.69	173.79	1298.60	0.00
1249	172.72	174.62	1298.61	0.00
1250	169.82	167.92	1298.54	0.00
1251	167.00	168.90	1298.55	0.00
1252	164.26	162.35	1298.48	0.00
1253	161.60	163.50	1298.49	0.00
1254	159.03	157.13	1298.43	0.00
1255	156.73	158.63	1298.44	0.00
1256	154.56	152.66	1298.38	0.00
1257	152.43	154.33	1298.40	0.00
1258	150.36	148.46	1298.34	0.00

1259	145.33	150.23	1298.35	0.00
1260	144.37	144.45	1298.30	0.00
1261	144.40	146.30	1298.31	0.00
1262	142.51	140.61	1298.26	0.00
1263	140.66	142.56	1298.28	0.00
1264	138.84	136.94	1298.22	0.00
1265	137.07	138.98	1298.24	0.00
1266	135.34	133.44	1298.18	0.00
1267	133.63	135.53	1298.20	0.00
1268	131.97	130.07	1298.15	0.00
1269	130.35	132.25	1298.17	0.00
1270	128.75	126.85	1298.11	0.00
1271	127.19	129.09	1298.14	0.00
1272	125.67	123.77	1298.08	0.00
1273	124.18	126.08	1298.11	0.00
1274	122.72	120.82	1298.05	0.00
1275	121.30	123.20	1298.08	0.00
1276	119.89	117.99	1298.02	0.00
1277	118.53	120.43	1298.05	0.00
1278	117.19	115.29	1297.99	0.00
1279	115.87	117.77	1298.02	0.00
1280	114.59	112.69	1297.96	0.00
1281	113.33	115.23	1297.99	0.00
1282	112.10	110.20	1297.93	0.00
1283	110.89	112.79	1297.96	0.00
1284	109.70	107.80	1297.89	0.00
1285	108.55	110.45	1297.93	0.00
1286	107.43	105.53	1297.86	0.00
1287	106.39	108.29	1297.90	0.00
1288	105.43	103.53	1297.84	0.00
1289	104.50	106.40	1297.87	0.00
1290	103.57	101.67	1297.81	0.00
1291	102.65	104.55	1297.85	0.00
1292	101.75	99.85	1297.79	0.00
1293	100.86	102.76	1297.83	0.00
1294	99.97	98.07	1297.76	0.00
1295	99.10	101.01	1297.80	0.00
1296	98.25	96.35	1297.74	0.00
1297	97.40	99.30	1297.78	0.00
1298	96.56	94.66	1297.72	0.00
1299	95.74	97.64	1297.76	0.00
1300	94.93	93.03	1297.70	0.00
1310	87.59	85.69	1297.60	0.00
1320	81.48	79.58	1297.52	0.00
1330	76.37	74.47	1297.45	0.00
1340	71.69	69.79	1297.39	0.00
1350	67.53	65.63	1297.33	0.00
1360	63.76	61.86	1297.28	0.00
1370	60.17	58.27	1297.23	0.00
1380	56.89	54.99	1297.19	0.00
1390	53.79	51.89	1297.15	0.00
1400	50.89	48.99	1297.11	0.00
1420	45.86	43.96	1297.04	0.00
1440	41.52	39.61	1296.97	0.00

Arc Travel Time Data Computed in WMS
Tue Aug 02 13:53:56 2005

BASIN 1A AREA 37.83 acres

ARC 23 Travel Time
8.00 min.

TYPE: LACDPW TC

EQN: $(10.0^{A-0.507}) * ((Cd*i)^{A-0.519}) * (L^{A0.483}) * (S^{A-0.135})$

L	Length	2081.96 ft
S	Slope	0.3564
%I	Percent impervious	1.00
SN	Soil number	293
RD	Rainfall depth	8.51
Cd	Soil runoff coefficient	0.80
i	Rainfall intensity	4.07

Time of Concentration for 1A 8.00 min.

BASIN 3A AREA 41.02 acres

ARC 22 Travel Time
9.00 min.

TYPE: LACDPW TC

EQN: $(10.0^{A-0.507}) * ((Cd*i)^{A-0.519}) * (L^{A0.483}) * (S^{A-0.135})$

L	Length	2523.32 ft
S	Slope	0.3107
%I	Percent impervious	1.00
SN	Soil number	293
RD	Rainfall depth	8.51
Cd	Soil runoff coefficient	0.79
i	Rainfall intensity	3.85

Time of Concentration for 3A 9.00 min.

BASIN 5A AREA 40.57 acres

ARC 26 Travel Time
8.00 min.

TYPE: LACDPW TC

EQN: $(10.0^{A-0.507}) * ((Cd*i)^{A-0.519}) * (L^{A0.483}) * (S^{A-0.135})$

L	Length	2128.74 ft
S	Slope	0.3711
%I	Percent impervious	1.00
SN	Soil number	293
RD	Rainfall depth	8.51
Cd	Soil runoff coefficient	0.80
i	Rainfall intensity	4.07

Time of Concentration for 5A 8.00 min.

 BASIN 7A AREA 30.39 acres

ARC 37 Travel Time

8.00 min.

TYPE: LACDPW TC

EQN: $(10.0 \wedge -0.507) * ((Cd*i) \wedge -0.519) * (L \wedge 0.483) * (S \wedge -0.135)$

L	Length	1941.62 ft
S	Slope	0.2112
%I	Percent impervious	1.00
SN	Soil number	291
RD	Rainfall depth	8.51
Cd	Soil runoff coefficient	0.84
i	Rainfall intensity	4.07

 Time of Concentration for 7A 8.00 min.

 BASIN 10B AREA 17.78 acres

ARC 13 Travel Time

6.00 min.

TYPE: LACDPW TC

EQN: $(10.0 \wedge -0.507) * ((Cd*i) \wedge -0.519) * (L \wedge 0.483) * (S \wedge -0.135)$

L	Length	1277.71 ft
S	Slope	0.2653
%I	Percent impervious	1.00
SN	Soil number	291
RD	Rainfall depth	8.51
Cd	Soil runoff coefficient	0.85
i	Rainfall intensity	4.66

 Time of Concentration for 10B 6.00 min.

 BASIN 12A AREA 34.81 acres

ARC 18 Travel Time

8.00 min.

TYPE: LACDPW TC

EQN: $(10.0 \wedge -0.507) * ((Cd*i) \wedge -0.519) * (L \wedge 0.483) * (S \wedge -0.135)$

L	Length	1723.78 ft
S	Slope	0.1833
%I	Percent impervious	1.00
SN	Soil number	291
RD	Rainfall depth	8.44
Cd	Soil runoff coefficient	0.84
i	Rainfall intensity	4.04

 Time of Concentration for 12A 8.00 min.

 BASIN 14A AREA 28.28 acres

ARC 11 Travel Time

8.00 min.

TYPE: LACDPW TC

EQN: $(10.0A-0.507) * ((Cd*i)^{A-0.519}) * (LA^{0.483}) * (SA^{-0.135})$

L	Length	1941.62 ft
S	Slope	0.1767
%I	Percent impervious	1.00
SN	Soil number	291
RD	Rainfall depth	8.33
Cd	Soil runoff coefficient	0.84
i	Rainfall intensity	3.98

Time of Concentration for 14A 8.00 min.

BASIN 16A AREA 23.76 acres

ARC 38 Travel Time

9.00 min.

TYPE: LACDPW TC

EQN: $(10.0A-0.507) * ((Cd*i)^{A-0.519}) * (LA^{0.483}) * (SA^{-0.135})$

L	Length	1834.36 ft
S	Slope	0.1172
%I	Percent impervious	1.00
SN	Soil number	291
RD	Rainfall depth	8.41
Cd	Soil runoff coefficient	0.83
i	Rainfall intensity	3.81

Time of Concentration for 16A 9.00 min.

BASIN 18A AREA 30.80 acres

ARC 27 Travel Time

10.00 min.

TYPE: LACDPW TC

EQN: $(10.0A-0.507) * ((Cd*i)^{A-0.519}) * (LA^{0.483}) * (SA^{-0.135})$

L	Length	2189.22 ft
S	Slope	0.1019
%I	Percent impervious	1.00
SN	Soil number	291
RD	Rainfall depth	8.49
Cd	Soil runoff coefficient	0.83
i	Rainfall intensity	3.66

Time of Concentration for 18A 10.00 min.

BASIN 20A AREA 31.98 acres

ARC 55 Travel Time

11.00 min.

TYPE: LACDPW TC

EQN: $(10.0A-0.507) * ((Cd*i)^{A-0.519}) * (LA^{0.483}) * (SA^{-0.135})$

L	Length	2490.25 ft
S	Slope	0.1000
%I	Percent impervious	1.00

1493EC10arctc.txt

SN Soil number 291
RD Rainfall depth 8.51
Cd Soil runoff coefficient 0.83
i Rainfall intensity 3.50

Time of Concentration for 20A 11.00 min.

BASIN 22A AREA 13.34 acres

ARC 28 Travel Time
6.00 min.

TYPE: LACDPW TC

EQN: $(10.0A-0.507) * ((Cd*i)^{A-0.519}) * (LA^{0.483}) * (S^{A-0.135})$

L Length 1200.21 ft
S Slope 0.1325
%I Percent impervious 1.00
SN Soil number 291
RD Rainfall depth 8.51
Cd Soil runoff coefficient 0.85
i Rainfall intensity 4.66

Time of Concentration for 22A 6.00 min.

BASIN 23C AREA 46.52 acres

ARC 24 Travel Time
9.00 min.

TYPE: LACDPW TC

EQN: $(10.0A-0.507) * ((Cd*i)^{A-0.519}) * (LA^{0.483}) * (S^{A-0.135})$

L Length 2332.88 ft
S Slope 0.2310
%I Percent impervious 1.00
SN Soil number 291
RD Rainfall depth 8.51
Cd Soil runoff coefficient 0.84
i Rainfall intensity 3.85

Time of Concentration for 23C 9.00 min.

BASIN 27C AREA 21.32 acres

ARC 25 Travel Time
9.00 min.

TYPE: LACDPW TC

EQN: $(10.0A-0.507) * ((Cd*i)^{A-0.519}) * (LA^{0.483}) * (S^{A-0.135})$

L Length 2255.38 ft
S Slope 0.2310
%I Percent impervious 1.00
SN Soil number 291
RD Rainfall depth 8.51
Cd Soil runoff coefficient 0.84
i Rainfall intensity 3.85

Time of Concentration for 27C 9.00 min.

BASIN 28D AREA 35.89 acres

ARC 35 Travel Time
9.00 min.

TYPE: LACDPW TC

EQN: $(10.0 \wedge -0.507) * ((Cd * i) \wedge -0.519) * (L \wedge 0.483) * (S \wedge -0.135)$

L	Length	2335.23 ft
S	Slope	0.2188
%I	Percent impervious	1.00
SN	Soil number	291
RD	Rainfall depth	8.51
Cd	Soil runoff coefficient	0.84
i	Rainfall intensity	3.85

Time of Concentration for 28D 9.00 min.

BASIN 30D AREA 33.06 acres

ARC 12 Travel Time
12.00 min.

TYPE: LACDPW TC

EQN: $(10.0 \wedge -0.507) * ((Cd * i) \wedge -0.519) * (L \wedge 0.483) * (S \wedge -0.135)$

L	Length	2924.96 ft
S	Slope	0.1180
%I	Percent impervious	1.00
SN	Soil number	291
RD	Rainfall depth	8.51
Cd	Soil runoff coefficient	0.83
i	Rainfall intensity	3.36

Time of Concentration for 30D 12.00 min.

BASIN 33A AREA 23.89 acres

ARC 14 Travel Time
9.00 min.

TYPE: LACDPW TC

EQN: $(10.0 \wedge -0.507) * ((Cd * i) \wedge -0.519) * (L \wedge 0.483) * (S \wedge -0.135)$

L	Length	1994.08 ft
S	Slope	0.1023
%I	Percent impervious	1.00
SN	Soil number	291
RD	Rainfall depth	8.51
Cd	Soil runoff coefficient	0.84
i	Rainfall intensity	3.85

Time of Concentration for 33A 9.00 min.

BASIN 36E AREA 47.95 acres

ARC 34 Travel Time
10.00 min.

TYPE: LACDPW TC

EQN: $(10.0 \wedge -0.507) * ((Cd * i) \wedge -0.519) * (LA \wedge 0.483) * (SA \wedge -0.135)$

L	Length	2808.70 ft
S	Slope	0.1901
%I	Percent impervious	1.00
SN	Soil number	291
RD	Rainfall depth	8.51
Cd	Soil runoff coefficient	0.83
i	Rainfall intensity	3.67

Time of Concentration for 36E 10.00 min.

BASIN 38F AREA 56.77 acres

ARC 44 Travel Time
10.00 min.

TYPE: LACDPW TC

EQN: $(10.0 \wedge -0.507) * ((Cd * i) \wedge -0.519) * (LA \wedge 0.483) * (SA \wedge -0.135)$

L	Length	2514.32 ft
S	Slope	0.1714
%I	Percent impervious	1.00
SN	Soil number	291
RD	Rainfall depth	8.51
Cd	Soil runoff coefficient	0.83
i	Rainfall intensity	3.67

Time of Concentration for 38F 10.00 min.

BASIN 40F AREA 41.49 acres

ARC 29 Travel Time
9.00 min.

TYPE: LACDPW TC

EQN: $(10.0 \wedge -0.507) * ((Cd * i) \wedge -0.519) * (LA \wedge 0.483) * (SA \wedge -0.135)$

L	Length	2103.69 ft
S	Slope	0.1155
%I	Percent impervious	1.00
SN	Soil number	291
RD	Rainfall depth	8.51
Cd	Soil runoff coefficient	0.84
i	Rainfall intensity	3.85

Time of Concentration for 40F 9.00 min.

BASIN 42A AREA 40.09 acres

ARC 32 Travel Time
9.00 min.

TYPE: LACDPW TC

EQN: $(10.0 \wedge -0.507) * ((Cd * i) \wedge -0.519) * (LA \wedge 0.483) * (SA \wedge -0.135)$

L	Length	2043.21 ft
S	Slope	0.1581

1493EC10arctc.txt

%I	Percent impervious	1.00
SN	Soil number	291
RD	Rainfall depth	8.51
Cd	Soil runoff coefficient	0.84
i	Rainfall intensity	3.85

Time of Concentration for 42A 9.00 min.

BASIN 44A AREA 41.55 acres

ARC 31 Travel Time

9.00 min.

TYPE: LACDPW TC

EQN: $(10.0 \wedge -0.507) * ((Cd * i) \wedge -0.519) * (L \wedge 0.483) * (S \wedge -0.135)$

L	Length	2090.96 ft
S	Slope	0.1345
%I	Percent impervious	1.00
SN	Soil number	297
RD	Rainfall depth	8.51
Cd	Soil runoff coefficient	0.80
i	Rainfall intensity	3.85

Time of Concentration for 44A 9.00 min.

BASIN 45G AREA 29.66 acres

ARC 30 Travel Time

8.00 min.

TYPE: LACDPW TC

EQN: $(10.0 \wedge -0.507) * ((Cd * i) \wedge -0.519) * (L \wedge 0.483) * (S \wedge -0.135)$

L	Length	1748.83 ft
S	Slope	0.1744
%I	Percent impervious	1.00
SN	Soil number	297
RD	Rainfall depth	8.51
Cd	Soil runoff coefficient	0.80
i	Rainfall intensity	4.07

Time of Concentration for 45G 8.00 min.

BASIN 47G AREA 33.81 acres

ARC 39 Travel Time

8.00 min.

TYPE: LACDPW TC

EQN: $(10.0 \wedge -0.507) * ((Cd * i) \wedge -0.519) * (L \wedge 0.483) * (S \wedge -0.135)$

L	Length	1814.99 ft
S	Slope	0.1868
%I	Percent impervious	2.98
SN	Soil number	297
RD	Rainfall depth	8.51
Cd	Soil runoff coefficient	0.80
i	Rainfall intensity	4.07

1493EC10arc.txt
8.00 min.

Time of Concentration for 47G

BASIN 49G AREA 28.41 acres

ARC 57 Travel Time
8.00 min.

TYPE: LACDPW TC
EQN: $(10.0A-0.507) * ((Cd*i)^{A-0.519}) * (LA^{0.483}) * (SA^{-0.135})$
L Length 1680.32 ft
S Slope 0.1660
%I Percent impervious 5.35
SN Soil number 297
RD Rainfall depth 8.51
Cd Soil runoff coefficient 0.81
i Rainfall intensity 4.07

Time of Concentration for 49G 8.00 min.

BASIN 53G AREA 27.35 acres

ARC 59 Travel Time
9.00 min.

TYPE: LACDPW TC
EQN: $(10.0A-0.507) * ((Cd*i)^{A-0.519}) * (LA^{0.483}) * (SA^{-0.135})$
L Length 2085.29 ft
S Slope 0.1506
%I Percent impervious 7.34
SN Soil number 297
RD Rainfall depth 8.51
Cd Soil runoff coefficient 0.80
i Rainfall intensity 3.85

Time of Concentration for 53G 9.00 min.

BASIN 55A AREA 37.38 acres

ARC 16 Travel Time
11.00 min.

TYPE: LACDPW TC
EQN: $(10.0A-0.507) * ((Cd*i)^{A-0.519}) * (LA^{0.483}) * (SA^{-0.135})$
L Length 2283.30 ft
S Slope 0.0720
%I Percent impervious 7.65
SN Soil number 297
RD Rainfall depth 8.51
Cd Soil runoff coefficient 0.80
i Rainfall intensity 3.51

Time of Concentration for 55A 11.00 min.

BASIN 58H AREA 18.36 acres

1493EC10arctc.txt

ARC 33 Travel Time
7.00 min.

TYPE: LACDPW TC

EQN: $(10.0 \wedge -0.507) * ((Cd * i) \wedge -0.519) * (L \wedge 0.483) * (S \wedge -0.135)$

L	Length	1566.42 ft
S	Slope	0.1705
%I	Percent impervious	23.32
SN	Soil number	97
RD	Rainfall depth	8.51
Cd	Soil runoff coefficient	0.81
i	Rainfall intensity	4.33

Time of Concentration for 58H 7.00 min.

Appendix 4
Existing Burned Watershed
2-Year, 24-hour event

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 11AS STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows include data points from 0 to 1400 minutes.

Total Runoff = 9.091 Acre-ft.
Peak Q = 124.62 CFS
Time to Peak = 1163 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 11RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows include data points from 0 to 1400 minutes.

Total Runoff = 9.108 Acre-ft.
Peak Q = 116.46 CFS
Time to Peak = 1166 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 32AC STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows include data points from 0 to 1145 minutes.

Total Runoff = 6.562 Acre-ft.
Peak Q = 67.45 CFS
Time to Peak = 1163 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 52RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Contains hydrograph data for 1400 minutes.

Total Runoff = 6.569 Acre-ft.
Peak Q = 63.41 CFS
Time to Peak = 1167 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 544RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Contains hydrograph data for 1400 minutes.

Total Runoff = 50.431 Acre-ft.
Peak Q = 297.16 CFS
Time to Peak = 1178 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 54RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Contains hydrograph data for 900 minutes.

Hydrograph table showing discharge (Q) in CFS versus time (TIME) for various intervals from 1190 to 1400 minutes.

Total Runoff = 49.554 Acre-ft.
Peak Q = 298.65 CFS
Time to Peak = 1185 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 59RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Hydrograph table for Modified Rational Method Hydrology, showing discharge (Q) vs. time (TIME) for a storm with a frequency of 50.

Total Runoff = 49.554 Acre-ft.
Peak Q = 298.65 CFS
Time to Peak = 1185 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 60I STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Hydrograph table for Modified Rational Method Hydrology, showing discharge (Q) vs. time (TIME) for a storm with a frequency of 60.

1290	0.47	1291	0.47	1292	0.47	1293	0.46	1294	0.46
1295	0.46	1296	0.46	1297	0.46	1298	0.45	1299	0.45
1300	0.45	1310	0.43	1320	0.41	1330	0.40	1340	0.39
1350	0.37	1360	0.36	1370	0.35	1380	0.34	1390	0.33
1400	0.32	1420	0.31	1440	0.30	1460	0.00	1500	0.00

Total Runoff = 1.932 Acre-ft.
 Peak Q = 36.31 CFS
 Time to Peak = 1158 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
 TOTAL HYDROGRAPH AT 1 65L STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

TIME	Q	TIME	Q	TIME	Q	TIME	Q	TIME	Q
0	0.00	100	0.25	200	0.26	300	0.28	400	0.30
500	0.33	600	0.37	700	0.43	800	0.50	900	0.63
1000	0.89	1050	1.18	1100	1.87	1110	2.16	1120	2.88
1130	4.16	1131	4.33	1132	4.52	1133	4.73	1134	4.95
1135	5.20	1136	5.45	1137	5.73	1138	6.04	1139	6.38
1140	6.76	1141	7.18	1142	7.51	1143	7.85	1144	8.22
1145	8.66	1146	9.15	1147	9.70	1148	10.35	1149	11.12
1150	12.04	1151	13.17	1152	14.56	1153	16.13	1154	18.34
1155	21.89	1156	32.38	1157	37.01	1158	37.39	1159	36.81
1160	35.60	1161	33.82	1162	31.46	1163	28.37	1164	24.09
1165	12.73	1166	7.77	1167	5.99	1168	4.78	1169	3.97
1170	3.37	1171	2.91	1172	2.55	1173	2.25	1174	2.09
1175	1.97	1176	1.86	1177	1.77	1178	1.69	1179	1.61
1180	1.55	1181	1.49	1182	1.43	1183	1.38	1184	1.34
1185	1.30	1186	1.26	1187	1.22	1188	1.19	1189	1.15
1190	1.12	1191	1.10	1192	1.07	1193	1.05	1194	1.02
1195	1.00	1196	0.98	1197	0.96	1198	0.94	1199	0.92
1200	0.90	1201	0.89	1202	0.87	1203	0.86	1204	0.84
1205	0.83	1206	0.82	1207	0.80	1208	0.79	1209	0.78
1210	0.77	1211	0.76	1212	0.74	1213	0.73	1214	0.72
1215	0.72	1216	0.71	1217	0.70	1218	0.69	1219	0.68
1220	0.67	1221	0.66	1222	0.65	1223	0.65	1224	0.64
1225	0.63	1226	0.63	1227	0.62	1228	0.62	1229	0.61
1230	0.60	1231	0.60	1232	0.59	1233	0.58	1234	0.58
1235	0.57	1236	0.57	1237	0.56	1238	0.56	1239	0.55
1240	0.55	1241	0.54	1242	0.54	1243	0.53	1244	0.53
1245	0.52	1246	0.52	1247	0.52	1248	0.51	1249	0.51
1250	0.50	1251	0.50	1252	0.50	1253	0.49	1254	0.49
1255	0.49	1256	0.48	1257	0.48	1258	0.48	1259	0.47
1260	0.47	1261	0.47	1262	0.46	1263	0.46	1264	0.46
1265	0.45	1266	0.45	1267	0.45	1268	0.45	1269	0.44
1270	0.44	1271	0.44	1272	0.43	1273	0.43	1274	0.43
1275	0.43	1276	0.42	1277	0.42	1278	0.42	1279	0.42
1280	0.41	1281	0.41	1282	0.41	1283	0.41	1284	0.41
1285	0.40	1286	0.40	1287	0.40	1288	0.40	1289	0.39
1290	0.39	1291	0.39	1292	0.39	1293	0.39	1294	0.38
1295	0.38	1296	0.38	1297	0.38	1298	0.38	1299	0.38
1300	0.37	1310	0.36	1320	0.34	1330	0.33	1340	0.32
1350	0.31	1360	0.30	1370	0.30	1380	0.28	1390	0.27
1400	0.26	1420	0.25	1440	0.24	1460	0.00	1500	0.00

Total Runoff = 1.786 Acre-ft.
 Peak Q = 37.39 CFS
 Time to Peak = 1158 Minutes

** LOS ANGELES COUNTY DEPARTMENT OF PUBLIC WORKS **
 ** MODIFIED RATIONAL METHOD HYDROLOGY **
 ** RESERVOIR ROUTING OUTPUT **

***** RESERVOIR ROUTING STORM DAY 4 *****

RESERVOIR ROUTING at 59AH STORM DAY 4 STORM FREQ. 50
 INITIAL WATER SURFACE ELEVATION: 1296.00
 RESERVOIR COMPOSITE ELEVATION-STORAGE-DISCHARGE DATA at 59AH

ELEVATION (ft.)	STORAGE (a.f.)	OUTFLOW (cfs)
1296.00	0.00	0.00
1297.00	0.00	40.90
1298.00	0.00	115.80
1299.00	0.00	212.80
1300.00	0.16	327.60
1301.00	0.33	457.80
1302.00	0.49	601.80
1303.00	0.65	758.30
1304.00	0.82	926.50
1305.00	0.98	1105.50
1306.00	1.15	1294.80
1307.00	1.31	1414.60
1308.00	1.47	1538.20
1309.00	1.64	1670.80
1310.00	1.80	1793.70

RESERVOIR DISCHARGE DATA: 59AH Known discharge 1

ELEVATION (ft.)	OUTFLOW (cfs)
1296.00	0.00
1297.00	40.90
1298.00	115.80
1299.00	212.80
1300.00	327.60
1301.00	457.80
1302.00	601.80
1303.00	758.30
1304.00	926.50
1305.00	1105.50
1306.00	1294.80
1307.00	1414.60
1308.00	1538.20
1309.00	1670.80
1310.00	1793.70

RESERVOIR ROUTING TABLE at 59AH

TIME	INFLOW (cfs)	OUTFLOW (cfs)	W.S.ELEV (ft.)	STORAGE (a.f.)
0	3.20	0.00	1296.00	0.00
100	4.75	1.55	1296.04	0.00
200	6.81	3.61	1296.09	0.00
300	8.05	4.85	1296.12	0.00
400	8.83	5.63	1296.14	0.00
500	9.52	6.32	1296.15	0.00
600	10.36	7.16	1296.18	0.00
700	11.51	8.31	1296.20	0.00
800	13.16	9.96	1296.24	0.00
900	15.85	12.65	1296.31	0.00
1000	21.34	18.14	1296.44	0.00
1050	27.58	24.38	1296.60	0.00
1100	44.04	40.84	1297.00	0.00
1110	48.95	45.75	1297.06	0.00
1120	55.26	52.06	1297.15	0.00
1130	64.39	61.19	1297.27	0.00

1131	65.50	68.70	1297.37	0.00
1132	66.65	63.45	1297.30	0.00
1133	67.83	71.03	1297.40	0.00
1134	69.04	65.84	1297.33	0.00
1135	70.31	73.51	1297.44	0.00
1136	71.64	68.44	1297.37	0.00
1137	73.04	76.24	1297.47	0.00
1138	74.52	71.32	1297.41	0.00
1139	76.07	79.27	1297.51	0.00
1140	77.74	74.54	1297.45	0.00
1141	79.50	82.70	1297.56	0.00
1142	81.38	78.18	1297.50	0.00
1143	83.24	86.44	1297.61	0.00
1144	85.22	82.02	1297.55	0.00
1145	87.35	90.55	1297.66	0.00
1146	89.63	86.43	1297.61	0.00
1147	92.10	95.30	1297.73	0.00
1148	94.75	91.55	1297.68	0.00
1149	97.62	100.82	1297.80	0.00
1150	100.77	97.57	1297.76	0.00
1151	104.25	107.45	1297.89	0.00
1152	108.17	104.97	1297.86	0.00
1153	112.55	115.75	1298.00	0.00
1154	117.58	114.38	1297.98	0.00
1155	123.99	127.19	1298.12	0.00
1156	136.48	133.28	1298.18	0.00
1157	144.62	147.82	1298.33	0.00
1158	150.19	146.99	1298.32	0.00
1159	156.40	159.60	1298.45	0.00
1160	164.20	161.00	1298.47	0.00
1161	173.50	176.70	1298.63	0.00
1162	183.44	180.24	1298.66	0.00
1163	193.61	196.81	1298.84	0.00
1164	204.14	200.94	1298.88	0.00
1165	214.49	214.42	1299.01	0.00
1166	223.64	217.49	1299.04	0.01
1167	226.54	222.52	1299.08	0.01
1168	233.54	227.49	1299.13	0.02
1169	240.46	233.78	1299.18	0.03
1170	245.83	239.98	1299.24	0.04
1171	250.75	245.48	1299.28	0.05
1172	255.42	250.51	1299.33	0.05
1173	259.93	255.25	1299.37	0.06
1174	264.69	259.92	1299.41	0.07
1175	269.80	264.77	1299.45	0.07
1176	275.07	269.84	1299.50	0.08
1177	280.23	275.01	1299.54	0.09
1178	285.06	280.06	1299.59	0.09
1179	289.37	284.79	1299.63	0.10
1180	292.98	289.01	1299.66	0.11
1181	295.79	292.57	1299.69	0.11
1182	297.74	295.34	1299.72	0.12
1183	298.82	297.29	1299.74	0.12
1184	299.07	298.39	1299.75	0.12
1185	298.49	298.65	1299.75	0.12
1186	297.10	298.08	1299.74	0.12
1187	294.92	296.71	1299.73	0.12
1188	292.00	294.56	1299.71	0.11
1189	288.38	291.67	1299.69	0.11
1190	284.12	288.08	1299.66	0.10
1191	279.32	283.88	1299.62	0.10
1192	274.07	279.13	1299.58	0.09
1193	268.50	273.94	1299.53	0.09
1194	262.73	268.43	1299.48	0.08

1195	256.85	262.72	1299.43	0.07
1196	250.97	256.89	1299.38	0.06
1197	245.17	251.06	1299.33	0.05
1198	239.52	245.30	1299.28	0.05
1199	234.06	239.67	1299.23	0.04
1200	228.82	234.23	1299.19	0.03
1201	223.85	229.01	1299.14	0.02
1202	219.07	224.01	1299.10	0.02
1203	214.55	219.25	1299.06	0.01
1204	210.22	214.71	1299.02	0.00
1205	206.04	205.41	1298.92	0.00
1206	202.04	202.66	1298.90	0.00
1207	198.13	197.51	1298.84	0.00
1208	194.28	194.91	1298.82	0.00
1209	190.49	189.86	1298.76	0.00
1210	186.76	187.38	1298.74	0.00
1211	183.01	182.39	1298.69	0.00
1212	179.25	179.87	1298.66	0.00
1213	175.52	174.90	1298.61	0.00
1214	171.76	172.39	1298.58	0.00
1215	167.99	167.36	1298.53	0.00
1216	164.23	164.86	1298.51	0.00
1217	160.44	159.81	1298.45	0.00
1218	156.69	157.31	1298.43	0.00
1219	152.93	152.31	1298.38	0.00
1220	149.17	149.79	1298.35	0.00
1221	145.41	144.78	1298.30	0.00
1222	141.73	142.35	1298.27	0.00
1223	138.07	137.45	1298.22	0.00
1224	134.45	135.07	1298.20	0.00
1225	130.92	130.29	1298.15	0.00
1226	127.46	128.08	1298.13	0.00
1227	124.07	123.45	1298.08	0.00
1228	120.76	121.38	1298.06	0.00
1229	117.53	116.90	1298.01	0.00
1230	114.36	114.99	1297.99	0.00
1231	111.28	110.66	1297.93	0.00
1232	108.29	108.91	1297.91	0.00
1233	105.37	104.75	1297.85	0.00
1234	102.53	103.15	1297.83	0.00
1235	99.78	99.15	1297.78	0.00
1236	97.11	97.73	1297.76	0.00
1237	94.52	93.90	1297.71	0.00
1238	92.01	92.63	1297.69	0.00
1239	89.57	88.95	1297.64	0.00
1240	87.22	87.84	1297.63	0.00
1241	84.94	84.32	1297.58	0.00
1242	82.74	83.36	1297.57	0.00
1243	80.61	79.99	1297.52	0.00
1244	78.56	79.18	1297.51	0.00
1245	76.57	75.95	1297.47	0.00
1246	74.66	75.28	1297.46	0.00
1247	72.81	72.19	1297.42	0.00
1248	71.02	71.65	1297.41	0.00
1249	69.30	68.67	1297.37	0.00
1250	67.64	68.27	1297.37	0.00
1251	66.04	65.42	1297.33	0.00
1252	64.48	65.10	1297.32	0.00
1253	62.96	62.34	1297.29	0.00
1254	61.49	62.12	1297.28	0.00
1255	60.06	59.44	1297.25	0.00
1256	58.68	59.30	1297.25	0.00
1257	57.33	56.71	1297.21	0.00
1258	56.03	56.65	1297.21	0.00

1259	54.77	54.15	1297.18	0.00
1260	53.60	54.22	1297.18	0.00
1261	52.46	51.84	1297.15	0.00
1262	51.36	51.98	1297.15	0.00
1263	50.29	49.67	1297.12	0.00
1264	49.25	49.88	1297.12	0.00
1265	48.28	47.65	1297.09	0.00
1266	47.36	47.98	1297.09	0.00
1267	46.46	45.84	1297.07	0.00
1268	45.58	46.20	1297.07	0.00
1269	44.72	44.09	1297.04	0.00
1270	43.89	44.51	1297.05	0.00
1271	43.10	42.48	1297.02	0.00
1272	42.33	42.95	1297.03	0.00
1273	41.56	40.94	1297.00	0.00
1274	40.80	41.43	1297.01	0.00
1275	40.06	39.44	1296.96	0.00
1276	39.33	39.95	1296.98	0.00
1277	38.61	37.99	1296.93	0.00
1278	37.90	38.53	1296.94	0.00
1279	37.21	36.59	1296.89	0.00
1280	36.53	37.16	1296.91	0.00
1281	35.87	35.24	1296.86	0.00
1282	35.22	35.84	1296.88	0.00
1283	34.58	33.96	1296.83	0.00
1284	33.96	34.58	1296.85	0.00
1285	33.35	32.73	1296.80	0.00
1286	32.76	33.38	1296.82	0.00
1287	32.18	31.56	1296.77	0.00
1288	31.62	32.24	1296.79	0.00
1289	31.07	30.45	1296.74	0.00
1290	30.54	31.16	1296.76	0.00
1291	30.03	29.40	1296.72	0.00
1292	29.53	30.16	1296.74	0.00
1293	29.06	28.43	1296.70	0.00
1294	28.60	29.22	1296.71	0.00
1295	28.16	27.54	1296.67	0.00
1296	27.74	28.36	1296.69	0.00
1297	27.34	26.71	1296.65	0.00
1298	26.97	27.59	1296.67	0.00
1299	26.63	26.01	1296.64	0.00
1300	26.29	26.92	1296.66	0.00
1310	23.22	23.85	1296.58	0.00
1320	20.69	21.31	1296.52	0.00
1330	18.78	19.40	1296.47	0.00
1340	17.17	17.79	1296.43	0.00
1350	15.80	16.43	1296.40	0.00
1360	14.71	15.34	1296.37	0.00
1370	13.80	14.43	1296.35	0.00
1380	13.01	13.63	1296.33	0.00
1390	12.34	12.96	1296.32	0.00
1400	11.76	12.38	1296.30	0.00
1420	10.80	11.43	1296.28	0.00
1440	10.06	10.68	1296.26	0.00

Appendix 5
Proposed Burned Watershed
2-Year, 24-hour event

1 130U	6.0	6.08	6.0	6.08	0.280	0	0	0.00000	0.00	0.00	0	220	6	3.30	0.03
1 131U	0.0	0.00	6.0	6.08	0.903	4	104	0.02343	3.00	0.00	0	220	0	3.30	0.00
1 132AU	6.0	6.00	661.0	334.88	61.399	4	408	0.03138	7.00	0.00	0	220	0	3.30	0.00
1 134V	29.4	34.14	29.4	34.14	2.580	0	0	0.00000	0.00	0.00	0	97	7	3.29	0.23
1 135V	0.0	0.00	29.4	34.14	2.637	5	138	0.01237	10.00	0.00	0	97	0	3.29	0.00
1 136AV	29.4	34.07	890.4	337.23	64.036	0	0	0.00000	0.00	0.00	0	97	0	3.29	0.00
1 37W	38.0	37.39	38.0	37.39	1.786	0	0	0.00000	0.00	0.00	0	297	9	3.30	0.01
1 38W	0.0	0.00	38.0	37.39	1.786	0	0	0.00000	0.00	0.00	0	297	0	3.30	0.00
1 39X	3.5	4.91	3.5	4.91	0.179	0	0	0.00000	0.00	0.00	0	297	5	3.30	0.03
1 140X	0.0	0.00	3.5	4.91	0.878	4	410	0.01225	3.00	0.00	0	297	0	3.30	0.00
1 142Y	5.2	7.81	5.2	7.81	0.655	0	0	0.00000	0.00	0.00	0	97	5	3.30	0.42
1 143Y	0.0	0.00	5.2	7.81	0.990	4	146	0.02238	3.00	0.00	0	97	0	3.30	0.00
1 144XY	5.2	7.63	8.7	11.90	1.859	0	84	0.01104	0.00	0.00	0	97	0	3.30	0.00
1 146Z	27.4	27.16	27.4	27.16	1.396	0	0	0.00000	0.00	0.00	0	297	9	3.30	0.03
1 147Z	0.0	0.00	27.4	27.16	1.612	4	120	0.02073	3.00	0.00	0	297	0	3.30	0.00
1 148XZ	27.4	27.04	36.1	38.85	3.470	0	0	0.00000	0.00	0.00	0	297	0	3.30	0.00

Normal End of MCDRBT

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 40A STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows show hydrograph data from 0 to 1400 minutes.

Total Runoff = 33.795 Acree-ft.
Peak Q = 226.16 CFS
Time to Peak = 1175 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 40RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows show hydrograph data from 0 to 1400 minutes.

Total Runoff = 31.195 Acree-ft.
Peak Q = 226.11 CFS
Time to Peak = 1175 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 53I STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows show hydrograph data from 0 to 1145 minutes.

1205	0.14	1206	0.14	1207	0.14	1208	0.14	1209	0.14
1210	0.14	1211	0.13	1212	0.13	1213	0.13	1214	0.13
1215	0.13	1216	0.13	1217	0.13	1218	0.13	1219	0.13
1220	0.12	1221	0.12	1222	0.12	1223	0.12	1224	0.12
1225	0.12	1226	0.12	1227	0.12	1228	0.12	1229	0.12
1230	0.12	1231	0.12	1232	0.11	1233	0.11	1234	0.11
1235	0.11	1236	0.11	1237	0.11	1238	0.11	1239	0.11
1240	0.11	1241	0.11	1242	0.11	1243	0.11	1244	0.11
1245	0.11	1246	0.11	1247	0.10	1248	0.10	1249	0.10
1250	0.10	1251	0.10	1252	0.10	1253	0.10	1254	0.10
1255	0.10	1256	0.10	1257	0.10	1258	0.10	1259	0.10
1260	0.10	1261	0.10	1262	0.10	1263	0.10	1264	0.10
1265	0.10	1266	0.10	1267	0.10	1268	0.10	1269	0.09
1270	0.09	1271	0.09	1272	0.09	1273	0.09	1274	0.09
1275	0.09	1276	0.09	1277	0.09	1278	0.09	1279	0.09
1280	0.09	1281	0.09	1282	0.09	1283	0.09	1284	0.09
1285	0.09	1286	0.09	1287	0.09	1288	0.09	1289	0.09
1290	0.09	1291	0.09	1292	0.09	1293	0.09	1294	0.09
1295	0.09	1296	0.09	1297	0.08	1298	0.08	1299	0.08
1300	0.08	1310	0.08	1320	0.08	1330	0.08	1340	0.08
1350	0.07	1360	0.07	1370	0.07	1380	0.07	1390	0.07
1400	0.07	1420	0.06	1440	0.06	1460	0.06	1500	0.00

Total Runoff = 0.253 Acre-Ft.
 Peak Q = 3.61 CFS
 Time to Peak = 1157 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
 TOTAL HYDROGRAPH AT 1 64RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

TIME	Q	TIME	Q	TIME	Q	TIME	Q
0	0.40	100	0.40	200	0.40	300	0.40
500	0.40	600	0.40	700	0.40	800	0.40
1000	0.40	1050	0.40	1100	0.40	1150	0.40
1130	0.48	1131	0.49	1132	0.50	1133	0.51
1135	0.54	1136	0.55	1137	0.57	1138	0.59
1140	0.63	1141	0.65	1142	0.67	1143	0.69
1145	0.75	1146	0.78	1147	0.82	1148	0.87
1150	0.98	1151	1.05	1152	1.14	1153	1.26
1155	1.69	1156	2.56	1157	3.00	1158	2.94
1160	2.39	1161	1.42	1162	0.81	1163	0.72
1165	0.54	1166	0.47	1167	0.44	1168	0.41
1170	0.40	1171	0.40	1172	0.40	1173	0.40
1175	0.40	1176	0.40	1177	0.40	1178	0.40
1180	0.40	1181	0.40	1182	0.40	1183	0.40
1185	0.40	1186	0.40	1187	0.40	1188	0.40
1190	0.40	1191	0.40	1192	0.40	1193	0.40
1195	0.40	1196	0.40	1197	0.40	1198	0.40
1200	0.40	1201	0.40	1202	0.40	1203	0.40
1205	0.40	1206	0.40	1207	0.40	1208	0.40
1210	0.40	1211	0.40	1212	0.40	1213	0.40
1215	0.40	1216	0.40	1217	0.40	1218	0.40
1220	0.40	1221	0.40	1222	0.40	1223	0.40
1225	0.40	1226	0.40	1227	0.40	1228	0.40
1230	0.40	1231	0.40	1232	0.40	1233	0.40
1235	0.40	1236	0.40	1237	0.40	1238	0.40
1240	0.40	1241	0.40	1242	0.40	1243	0.40
1245	0.40	1246	0.40	1247	0.40	1248	0.40
1250	0.40	1251	0.40	1252	0.40	1253	0.40
1255	0.40	1256	0.40	1257	0.40	1258	0.40
1260	0.40	1261	0.40	1262	0.40	1263	0.40
1265	0.40	1266	0.40	1267	0.40	1268	0.40
1270	0.40	1271	0.40	1272	0.40	1273	0.40
1275	0.40	1276	0.40	1277	0.40	1278	0.40
1280	0.40	1281	0.40	1282	0.40	1283	0.40
1285	0.40	1286	0.40	1287	0.40	1288	0.40
1290	0.40	1291	0.40	1292	0.40	1293	0.40
1295	0.40	1296	0.40	1297	0.40	1298	0.40
1300	0.40	1310	0.40	1320	0.40	1330	0.40
1350	0.40	1360	0.40	1370	0.40	1380	0.40
1400	0.40	1420	0.40	1440	0.40	1460	0.40

Total Runoff = 0.869 Acre-fr.
 Peak Q = 3.00 CFS
 Time to Peak = 1157 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
 TOTAL HYDROGRAPH AT 1 78M STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

TIME	Q	TIME	Q	TIME	Q	TIME	Q
0	0.05	100	0.05	200	0.05	300	0.05
500	0.05	600	0.05	700	0.05	800	0.05
1000	0.18	1050	0.30	1100	0.55	1150	0.84
1130	0.97	1131	1.00	1132	1.03	1133	1.06
1135	1.14	1136	1.15	1137	1.22	1138	1.28
1140	1.33	1141	1.43	1142	1.48	1143	1.54
1145	1.69	1146	1.78	1147	1.88	1148	2.00
1150	2.32	1151	2.32	1152	2.76	1153	3.09
1155	4.27	1156	6.97	1157	7.73	1158	7.55
1160	5.99	1161	3.10	1162	1.82	1163	1.48
1165	1.09	1166	0.92	1167	0.86	1168	0.78
1170	0.67	1171	0.63	1172	0.59	1173	0.56
1175	0.50	1176	0.48	1177	0.46	1178	0.44
1180	0.41	1181	0.39	1182	0.36	1183	0.34
1185	0.31	1186	0.30	1187	0.28	1188	0.27
1190	0.25	1191	0.24	1192	0.23	1193	0.22
1195	0.20	1196	0.20	1197	0.19	1198	0.18
1200	0.17	1201	0.16	1202	0.16	1203	0.16
1205	0.15	1206	0.15	1207	0.15	1208	0.15
1210	0.14	1211	0.14	1212	0.14	1213	0.14
1215	0.13	1216	0.13	1217	0.13	1218	0.13
1220	0.13	1221	0.12	1222	0.12	1223	0.12
1225	0.12	1226	0.12	1227	0.12	1228	0.11
1230	0.11	1231	0.11	1232	0.11	1233	0.11
1235	0.11	1236	0.11	1237	0.10	1238	0.10
1240	0.10	1241	0.10	1242	0.10	1243	0.10
1245	0.10	1246	0.10	1247	0.10	1248	0.10
1250	0.09	1251	0.09	1252	0.09	1253	0.09
1255	0.09	1256	0.09	1257	0.09	1258	0.09

Total Runoff = 0.229 Acre-ft.
Peak Q = 5.18 CFS
Time to Peak = 1157 Minutes

UNADJUSTED RATIONAL METHOD HYDROLOGY
HYDROGRAPH AT 1 72RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Data points range from 0 to 1400 minutes, showing a peak flow of 5.18 CFS at 1157 minutes.

Total Runoff = 0.894 Acre-ft.
Peak Q = 5.09 CFS
Time to Peak = 1158 Minutes

ADJUSTED RATIONAL METHOD HYDROLOGY
HYDROGRAPH AT 1 750 STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Data points range from 0 to 1400 minutes, showing a peak flow of 13.57 CFS at 1157 minutes.

Total Runoff = 0.430 Acre-ft.
Peak Q = 13.57 CFS
Time to Peak = 1157 Minutes

ADJUSTED RATIONAL METHOD HYDROLOGY
HYDROGRAPH AT 1 10RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Data points range from 0 to 500 minutes, showing a peak flow of 1.00 CFS at 500 minutes.

TOTAL HYDROGRAPH AT 1 136AV STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows from 0 to 1400 minutes.

Total Runoff = 64.036 Acres-ft.
Peak Q = 337.23 CFS
Time to Peak = 1175 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY

TOTAL HYDROGRAPH AT 1 136AV STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows from 0 to 1400 minutes.

Total Runoff = 1.786 Acres-ft.
Peak Q = 37.35 CFS
Time to Peak = 1152 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY

TOTAL HYDROGRAPH AT 1 140X STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows from 0 to 1350 minutes.

Total Runoff = 3.470 Acre-ft.
Peak Q = 38.85 CFS
Time to Peak = 1158 Minutes

Appendix 5
Proposed Burned Watershed
50-Year, 24-hour event

Los Angeles County Flood Control District
Modified Rational Method Hydrology

Table with columns: LOCATION, SUBAREA, STORM DAY 4, STORM FREQUENCY 50, CONV, CONV, CONV, CONV, CONV, CONTROL, SOIL, RAIN, FCT. Rows include locations like 1 1A, 1 2A, 1 3A, etc.

1 130U	6.0	20.48	6.0	20.48	1.183	0	0	0.00000	0.00	0.00	0	220	6	8.51	0.03
1 131U	0.0	0.00	6.0	20.48	1.438	4	104	0.02343	3.00	0.00	0	220	0	8.51	0.00
1 132AU	6.0	20.26	961.0	1410.34	258.803	4	408	0.03138	7.00	0.00	0	220	0	8.51	0.00
1 134V	29.4	102.31	29.4	102.31	8.397	0	0	0.00000	0.00	0.00	0	97	7	8.51	0.23
1 135V	0.0	0.00	29.4	102.31	8.456	5	138	0.01237	10.00	0.00	0	97	0	8.51	0.00
136AV	29.4	102.00	890.4	1423.88	267.259	0	0	0.00000	0.00	0.00	0	97	0	8.51	0.00
37W	38.0	116.68	38.0	116.68	8.501	0	0	0.00000	0.00	0.00	0	297	9	8.51	0.01
38W	0.0	0.00	38.0	116.68	8.501	0	0	0.00000	0.00	0.00	0	297	0	8.51	0.00
1 139X	3.5	14.64	3.5	14.64	0.812	0	0	0.00000	0.00	0.00	0	297	5	8.51	0.03
1 140X	0.0	0.00	3.5	14.64	1.232	4	410	0.01225	3.00	0.00	0	297	0	8.51	0.00
1 142Y	5.2	22.10	5.2	22.10	1.923	0	0	0.00000	0.00	0.00	0	97	5	8.51	0.42
1 143Y	0.0	0.00	5.2	22.10	1.979	4	146	0.02238	3.00	0.00	0	97	0	8.51	0.00
1 144XY	5.2	21.69	8.7	35.41	3.211	0	84	0.01104	0.00	0.00	0	97	0	8.51	0.00
1 146Z	27.4	84.35	27.4	84.35	6.353	0	0	0.00000	0.00	0.00	0	297	9	8.51	0.03
1 147Z	0.0	0.00	27.4	84.35	6.410	4	120	0.02073	3.00	0.00	0	297	0	8.51	0.00
1 148XZ	27.4	83.90	36.1	119.30	9.620	0	0	0.00000	0.00	0.00	0	297	0	8.51	0.00

Normal End of MODRAT

1493EC600.sum

Hydrograph data for 1493EC600.sum showing time intervals from 1150 to 1400 minutes with corresponding flow values (Q) and time points.

Total Runoff = 2.941 Acre-ft.
Peak Q = 40.13 CFS
Time to Peak = 1157 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 53RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Hydrograph data for Modified Rational Method at 53RT, showing time intervals from 0 to 1400 minutes with corresponding flow values (Q) and time points.

Total Runoff = 2.999 Acre-ft.
Peak Q = 39.84 CFS
Time to Peak = 1157 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 64K STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Hydrograph data for Modified Rational Method at 64K, showing time intervals from 0 to 1200 minutes with corresponding flow values (Q) and time points.

Table with 9 columns showing time (0 to 1400) and flow rate (0.24 to 0.59) for a hydrograph.

Total Runoff = 1.630 Acre-ft.
Peak Q = 22.24 CFS
Time to Peak = 1157 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 70RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns showing time (0 to 1400) and flow rate (0.40 to 12.10) for a hydrograph.

Total Runoff = 1.762 Acre-ft.
Peak Q = 21.84 CFS
Time to Peak = 1158 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 72N STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns showing time (0 to 1400) and flow rate (0.14 to 11.62) for a hydrograph.

Total Runoff = 1.087 Acre-Ft.
Peak Q = 14.83 CFS
Time to Peak = 1157 Minutes

UNITED RATIONAL METHOD HYDROLOGY
HYDROGRAPH AT 1 72RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows range from 0 to 1400 minutes.

Total Runoff = 1.347 Acre-Ft.
Peak Q = 14.63 CFS
Time to Peak = 1157 Minutes

UNITED RATIONAL METHOD HYDROLOGY
HYDROGRAPH AT 1 750 STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows range from 0 to 1400 minutes.

Total Runoff = 2.845 Acre-Ft.
Peak Q = 38.82 CFS
Time to Peak = 1157 Minutes

UNITED RATIONAL METHOD HYDROLOGY
HYDROGRAPH AT 1 10RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows range from 0 to 1400 minutes.

TOTAL HYDROGRAPH AT 1 140X STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. It lists hydrograph data points from 0 to 1400 minutes.

Total Runoff = 0.812 Acree-ft.
Peak Q = 14.64 CFS
Time to Peak = 1157 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 140RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. It lists hydrograph data points from 0 to 1400 minutes for the modified rational method.

Total Runoff = 1.232 Acree-ft.
Peak Q = 14.13 CFS
Time to Peak = 1158 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 143Y STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. It lists hydrograph data points from 0 to 1400 minutes for the modified rational method at station 143Y.

1265	2.65	1266	2.64	1267	2.63	1268	2.61	1269	2.60
1270	2.58	1271	2.57	1272	2.55	1273	2.54	1274	2.53
1275	2.52	1276	2.51	1277	2.49	1278	2.48	1279	2.47
1280	2.46	1281	2.45	1282	2.44	1283	2.42	1284	2.41
1285	2.40	1286	2.39	1287	2.38	1288	2.37	1289	2.36
1290	2.35	1291	2.34	1292	2.33	1293	2.32	1294	2.31
	2.30	1296	2.29	1297	2.28	1298	2.27	1299	2.27
	2.25	1310	2.18	1320	2.10	1330	2.04	1340	1.98
	1.92	1360	1.87	1370	1.83	1380	1.79	1390	1.75
1400	1.72	1420	1.65	1440	1.59	1460	1.22	1500	1.20

Total Runoff = 9.620 Acres-ft.
Peak Q = 119.30 CFS
Time to Peak = 1156 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY

TOTAL HYDROGRAPH AT 1 40A STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 11 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows represent time intervals from 2:00 to 14:00 with corresponding discharge values.

Total Runoff = 160.465 Acres-ft.
Peak Q = 948.66 CFS
Time to Peak = 1171 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY

TOTAL HYDROGRAPH AT 1 40RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 11 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows represent time intervals from 2:00 to 14:00 with corresponding discharge values.

Total Runoff = 160.465 Acres-ft.
Peak Q = 948.07 CFS
Time to Peak = 1171 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY

TOTAL HYDROGRAPH AT 1 53E STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 11 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows represent time intervals from 0:00 to 1:45 with corresponding discharge values.

Appendix 6
Proposed Burned Watershed
Peak Mitigated Flow - Q_{pm}

1 130U	6.0	0.61	6.0	0.61	0.036	0	0	0.00000	0.00	0.00	0	220	6	0.75	0.03
1 131U	0.0	0.00	6.0	0.61	0.827	4	104	0.02343	3.00	0.00	0	220	0	0.75	0.00
1 132AU	6.0	0.60	861.0	1355.42	233.920	4	408	0.03138	7.00	0.00	0	220	0	0.75	0.00
1 134V	29.4	102.31	29.4	102.31	8.397	0	0	0.00000	0.00	0.00	0	97	7	8.51	0.23
1 135V	0.0	0.00	29.4	102.31	8.456	5	138	0.01237	10.00	0.00	0	97	0	8.51	0.00
1 136AV	29.4	102.00	890.4	1380.04	242.376	0	0	0.00000	0.00	0.00	0	97	0	8.51	0.00
1 37W	38.0	116.68	38.0	116.68	8.501	0	0	0.00000	0.00	0.00	0	297	9	8.51	0.01
1 38W	0.0	0.00	38.0	116.68	8.501	0	0	0.00000	0.00	0.00	0	297	0	8.51	0.00
1 139K	3.5	14.64	3.5	14.64	0.812	0	0	0.00000	0.00	0.00	0	297	5	8.51	0.03
1 140X	0.0	0.00	3.5	14.64	1.232	4	410	0.01225	3.00	0.00	0	297	0	8.51	0.00
1 142Y	5.2	1.22	5.2	1.22	0.140	0	0	0.00000	0.00	0.00	0	97	5	0.75	0.42
1 143Y	0.0	0.00	5.2	1.22	0.832	4	146	0.02238	3.00	0.00	0	97	0	0.75	0.00
1 144XY	5.2	1.19	8.7	15.26	2.064	0	84	0.01104	0.00	0.00	0	97	0	0.75	0.00
1 146Z	27.4	84.35	27.4	84.35	6.353	0	0	0.00000	0.00	0.00	0	297	9	8.51	0.53
1 147Z	0.0	0.00	27.4	84.35	6.410	4	120	0.02073	3.00	0.00	0	297	0	8.51	0.00
1 148XZ	27.4	83.90	36.1	99.07	8.473	0	0	0.00000	0.00	0.00	0	297	0	8.51	0.60

Normal End of MODRAT

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 40A STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows range from 0 to 1400 minutes.

Total Runoff = 160.465 Acre-Ft.
Peak Q = 948.08 CFS
Time to Peak = 1171 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 40RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows range from 0 to 1400 minutes.

Total Runoff = 160.465 Acre-ft.
Peak Q = 948.07 CFS
Time to Peak = 1171 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 53I STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows range from 0 to 1145 minutes.

Hydrology data table with columns for Time, Q, and values for various time intervals from 1260 to 1400.

Total Runoff = 1.630 Acra-ft.
Peak Q = 22.24 CFS
Time to Peak = 1157 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY

TOTAL HYDROGRAPH AT 1 70RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Detailed hydrology data table with columns for Time, Q, and values from 0 to 1400.

Total Runoff = 1.782 Acra-ft.
Peak Q = 21.84 CFS
Time to Peak = 1158 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY

TOTAL HYDROGRAPH AT 1 72N STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Detailed hydrology data table with columns for Time, Q, and values from 0 to 1400.

Total Runoff = 1.087 Acre-ft.
Peak Q = 14.83 CFS
Time to Peak = 1157 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
HYDROGRAPH AT 1 72RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows range from 0 to 1400 minutes.

Total Runoff = 1.347 Acre-ft.
Peak Q = 14.63 CFS
Time to Peak = 1157 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
HYDROGRAPH AT 1 750 STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows range from 0 to 1400 minutes.

Total Runoff = 2.845 Acre-ft.
Peak Q = 33.32 CFS
Time to Peak = 1157 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
HYDROGRAPH AT 1 75RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows range from 0 to 500 minutes.

Hydrograph data for 1493EC615.sum showing discharge (Q) in CFS over time from 1180 to 1400 minutes. Values range from 0.46 to 4.87 CFS.

Total Runoff = 2.975 Acre-ft.
Peak Q = 31.18 CFS
Time to Peak = 1163 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 104Q STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Hydrograph data for 1 104Q showing discharge (Q) in CFS over time from 0 to 1400 minutes. Values range from 0.60 to 33.32 CFS.

Total Runoff = 12.554 Acre-ft.
Peak Q = 150.19 CFS
Time to Peak = 1158 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 104RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Hydrograph data for 1 104RT showing discharge (Q) in CFS over time from 0 to 1400 minutes. Values range from 0.40 to 39.47 CFS.

TOTAL HYDROGRAPH AT 1 136AV STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows show discharge values from 0 to 1400 over time.

Total Runoff = 242.376 Acra-Ft.
Peak Q = 1380.04 CFS
Time to Peak = 1170 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 136AV STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows show discharge values from 0 to 1400 over time.

Total Runoff = 3.501 Acra-Ft.
Peak Q = 116.68 CFS
Time to Peak = 1133 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 140X STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

Table with 10 columns: TIME, Q, TIME, Q, TIME, Q, TIME, Q, TIME, Q. Rows show discharge values from 0 to 1400 over time.

1210	0.08	1211	0.08	1212	0.08	1213	0.08	1214	0.08
1215	0.08	1216	0.08	1217	0.08	1218	0.08	1219	0.08
1220	0.08	1221	0.08	1222	0.08	1223	0.08	1224	0.08
1225	0.07	1226	0.07	1227	0.07	1228	0.07	1229	0.07
1230	0.07	1231	0.07	1232	0.07	1233	0.07	1234	0.07
1235	0.07	1236	0.07	1237	0.07	1238	0.07	1239	0.07
	0.07	1241	0.07	1242	0.07	1243	0.07	1244	0.07
	0.07	1246	0.07	1247	0.07	1248	0.06	1249	0.06
1250	0.06	1251	0.06	1252	0.06	1253	0.06	1254	0.06
1255	0.06	1256	0.06	1257	0.06	1258	0.06	1259	0.06
1260	0.06	1261	0.06	1262	0.06	1263	0.06	1264	0.06
1265	0.06	1266	0.06	1267	0.06	1268	0.06	1269	0.06
1270	0.06	1271	0.06	1272	0.06	1273	0.06	1274	0.06
1275	0.06	1276	0.06	1277	0.06	1278	0.06	1279	0.06
1280	0.06	1281	0.06	1282	0.06	1283	0.06	1284	0.06
1285	0.06	1286	0.06	1287	0.06	1288	0.05	1289	0.05
1290	0.05	1291	0.05	1292	0.05	1293	0.05	1294	0.05
1295	0.05	1296	0.05	1297	0.05	1298	0.05	1299	0.05
1300	0.05	1310	0.05	1320	0.05	1330	0.05	1340	0.05
1350	0.05	1360	0.04	1370	0.04	1380	0.04	1390	0.04
1400	0.04	1420	0.04	1440	0.04	1460	0.00	1500	0.00

Total Runoff = 0.140 Acre-Ft.
Peak Q = 1.22 CFS
Time to Peak = 1157 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 143RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

TIME	Q	TIME	Q	TIME	Q	TIME	Q	TIME	Q
0	0.40	100	0.40	200	0.40	300	0.40	400	0.40
500	0.40	600	0.40	700	0.40	800	0.40	900	0.40
1000	0.40	1050	0.40	1100	0.40	1110	0.40	1120	0.40
1130	0.40	1131	0.40	1132	0.40	1133	0.40	1134	0.40
1135	0.40	1136	0.40	1137	0.40	1138	0.40	1139	0.40
1140	0.40	1141	0.40	1142	0.40	1143	0.40	1144	0.40
1145	0.40	1146	0.40	1147	0.40	1148	0.40	1149	0.40
1150	0.40	1151	0.40	1152	0.41	1153	0.43	1154	0.47
1155	0.55	1156	0.77	1157	1.07	1158	1.19	1159	1.13
1160	0.99	1161	0.74	1162	0.49	1163	0.43	1164	0.41
1165	0.40	1166	0.40	1167	0.40	1168	0.40	1169	0.40
1170	0.40	1171	0.40	1172	0.40	1173	0.40	1174	0.40
1175	0.40	1176	0.40	1177	0.40	1178	0.40	1179	0.40
1180	0.40	1181	0.40	1182	0.40	1183	0.40	1184	0.40
1185	0.40	1186	0.40	1187	0.40	1188	0.40	1189	0.40
1190	0.40	1191	0.40	1192	0.40	1193	0.40	1194	0.40
1195	0.40	1196	0.40	1197	0.40	1198	0.40	1199	0.40
1200	0.40	1201	0.40	1202	0.40	1203	0.40	1204	0.40
1205	0.40	1206	0.40	1207	0.40	1208	0.40	1209	0.40
1210	0.40	1211	0.40	1212	0.40	1213	0.40	1214	0.40
1215	0.40	1216	0.40	1217	0.40	1218	0.40	1219	0.40
1220	0.40	1221	0.40	1222	0.40	1223	0.40	1224	0.40
1225	0.40	1226	0.40	1227	0.40	1228	0.40	1229	0.40
1230	0.40	1231	0.40	1232	0.40	1233	0.40	1234	0.40
	0.40	1236	0.40	1237	0.40	1238	0.40	1239	0.40
	0.40	1241	0.40	1242	0.40	1243	0.40	1244	0.40
1245	0.40	1246	0.40	1247	0.40	1248	0.40	1249	0.40
1250	0.40	1251	0.40	1252	0.40	1253	0.40	1254	0.40
1255	0.40	1256	0.40	1257	0.40	1258	0.40	1259	0.40
1260	0.40	1261	0.40	1262	0.40	1263	0.40	1264	0.40
1265	0.40	1266	0.40	1267	0.40	1268	0.40	1269	0.40
1270	0.40	1271	0.40	1272	0.40	1273	0.40	1274	0.40
1275	0.40	1276	0.40	1277	0.40	1278	0.40	1279	0.40
1280	0.40	1281	0.40	1282	0.40	1283	0.40	1284	0.40
1285	0.40	1286	0.40	1287	0.40	1288	0.40	1289	0.40
1290	0.40	1291	0.40	1292	0.40	1293	0.40	1294	0.40
1295	0.40	1296	0.40	1297	0.40	1298	0.40	1299	0.40
1300	0.40	1310	0.40	1320	0.40	1330	0.40	1340	0.40
1350	0.40	1360	0.40	1370	0.40	1380	0.40	1390	0.40
1400	0.40	1420	0.40	1440	0.40	1460	0.40	1500	0.40

Total Runoff = 0.832 Acre-Ft.
Peak Q = 1.19 CFS
Time to Peak = 1159 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
TOTAL HYDROGRAPH AT 1 147Z STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

TIME	Q	TIME	Q	TIME	Q	TIME	Q	TIME	Q
0	0.00	100	0.79	200	0.85	300	0.92	400	1.00
500	1.11	600	1.25	700	1.45	800	1.79	900	2.67
1000	4.74	1050	6.58	1100	10.53	1110	11.89	1120	13.83
1130	15.75	1131	17.09	1132	17.46	1133	17.85	1134	18.26
1135	16.73	1136	19.26	1137	19.70	1138	20.26	1139	20.65
1140	21.51	1141	22.22	1142	23.00	1143	23.85	1144	24.79
1145	25.86	1146	27.05	1147	28.29	1148	29.67	1149	31.26
1150	33.15	1151	35.41	1152	38.23	1153	41.67	1154	46.25
1155	53.29	1156	75.01	1157	83.65	1158	84.35	1159	83.29
1160	81.06	1161	77.77	1162	73.16	1163	66.87	1164	57.96
1165	34.55	1166	23.66	1167	20.16	1168	17.96	1169	16.38
1170	16.04	1171	13.92	1172	13.00	1173	12.23	1174	11.57
1175	11.01	1176	10.49	1177	9.97	1178	9.49	1179	9.07
1180	8.70	1181	8.34	1182	8.03	1183	7.75	1184	7.48
1185	7.24	1186	7.02	1187	6.81	1188	6.61	1189	6.43
1190	6.26	1191	6.13	1192	5.95	1193	5.82	1194	5.68
1195	5.54	1196	5.44	1197	5.29	1198	5.13	1199	4.96
1200	4.84	1201	4.76	1202	4.57	1203	4.44	1204	4.34
1205	4.22	1206	4.12	1207	4.00	1208	3.81	1209	3.62
1210	3.73	1211	3.54	1212	3.55	1213	3.48	1214	3.39
1215	3.32	1216	3.25	1217	3.18	1218	3.12	1219	3.05
1220	2.99	1221	2.85	1222	2.87	1223	2.82	1224	2.76
1225	2.72	1226	2.65	1227	2.61	1228	2.57	1229	2.52
1230	2.48	1231	2.47	1232	2.39	1233	2.35	1234	2.30
	2.27	1236	2.25	1237	2.20	1238	2.16	1239	2.12
1245	1.99	1241	1.95	1242	1.93	1243	1.90	1244	1.86
1250	1.83	1247	1.82	1248	1.78	1249	1.76	1250	1.73
1255	1.70	1252	1.69	1253	1.66	1254	1.64	1255	1.62
1260	1.60	1256	1.57	1257	1.55	1258	1.53	1259	1.51
1265	1.52	1261	1.51	1262	1.49	1263	1.48	1264	1.47

1265	1.56	1266	1.55	1267	1.54	1268	1.53	1269	1.52
1270	1.51	1271	1.50	1272	1.49	1273	1.48	1274	1.47
1275	1.46	1276	1.45	1277	1.44	1278	1.43	1279	1.42
1280	1.41	1281	1.40	1282	1.39	1283	1.39	1284	1.38
1285	1.37	1286	1.36	1287	1.35	1288	1.35	1289	1.34
1290	1.33	1291	1.32	1292	1.32	1293	1.31	1294	1.30
	1.29	1296	1.28	1297	1.28	1298	1.27	1299	1.26
	1.25	1310	1.20	1326	1.14	1330	1.09	1340	1.05
	1.00	1360	0.97	1370	0.93	1380	0.91	1390	0.88
1400	0.85	1420	0.80	1440	0.76	1460	0.60	1500	0.00

Total Runoff = 6.353 Acre-ft.
 Peak Q = 84.35 CFS
 Time to Peak = 1156 Minutes

MODIFIED RATIONAL METHOD HYDROLOGY
 TOTAL HYDROGRAPH AT 1 147RT STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

TIME	Q	TIME	Q	TIME	Q	TIME	Q	TIME	Q
0	0.40	100	0.79	200	0.85	300	0.91	400	1.00
500	1.11	600	1.26	700	1.45	800	1.78	900	2.67
1000	4.73	1050	6.54	1100	10.50	1110	11.85	1120	13.77
1130	18.66	1131	16.99	1132	17.35	1133	17.74	1134	18.15
1135	18.60	1136	19.38	1137	19.57	1138	20.11	1139	20.69
1140	21.33	1141	22.03	1142	22.79	1143	23.63	1144	24.54
1145	25.56	1146	26.75	1147	27.98	1148	29.33	1149	30.97
1150	32.69	1151	34.87	1152	37.55	1153	40.67	1154	45.18
1155	51.66	1156	69.64	1157	82.83	1158	83.90	1159	83.81
1160	81.43	1161	78.62	1162	74.14	1163	68.32	1164	59.96
1165	40.52	1166	25.40	1167	21.31	1168	18.36	1169	16.90
1170	15.37	1171	14.26	1172	13.26	1173	12.47	1174	11.77
1175	11.18	1176	10.65	1177	10.15	1178	9.65	1179	9.21
1180	8.92	1181	8.46	1182	8.14	1183	7.95	1184	7.57
1185	7.33	1186	7.10	1187	6.88	1188	6.68	1189	6.50
1190	6.32	1191	6.16	1192	6.01	1193	5.86	1194	5.73
1195	5.60	1196	5.48	1197	5.35	1198	5.20	1199	5.04
1200	4.89	1201	4.75	1202	4.62	1203	4.49	1204	4.38
1205	4.27	1206	4.16	1207	4.05	1208	3.95	1209	3.86
1210	3.77	1211	3.68	1212	3.59	1213	3.51	1214	3.43
1215	3.35	1216	3.29	1217	3.22	1218	3.15	1219	3.08
1220	3.01	1221	2.95	1222	2.89	1223	2.84	1224	2.79
1225	2.73	1226	2.68	1227	2.63	1228	2.59	1229	2.54
1230	2.50	1231	2.45	1232	2.41	1233	2.37	1234	2.32
1235	2.28	1236	2.25	1237	2.21	1238	2.18	1239	2.14
1240	2.11	1241	2.07	1242	2.04	1243	2.01	1244	1.99
1245	1.95	1246	1.91	1247	1.89	1248	1.86	1249	1.83
1250	1.81	1251	1.78	1252	1.76	1253	1.73	1254	1.71
1255	1.69	1256	1.68	1257	1.67	1258	1.65	1259	1.64
1260	1.63	1261	1.62	1262	1.60	1263	1.59	1264	1.58
1265	1.57	1266	1.56	1267	1.55	1268	1.54	1269	1.53
1270	1.51	1271	1.50	1272	1.49	1273	1.48	1274	1.47
1275	1.46	1276	1.45	1277	1.44	1278	1.44	1279	1.43
1280	1.42	1281	1.41	1282	1.40	1283	1.39	1284	1.38
1285	1.37	1286	1.37	1287	1.36	1288	1.35	1289	1.34
	1.34	1291	1.33	1292	1.32	1293	1.31	1294	1.30
	1.30	1296	1.29	1297	1.28	1298	1.27	1299	1.27
	1.26	1310	1.20	1320	1.14	1330	1.09	1340	1.05
1350	1.01	1360	0.97	1370	0.93	1380	0.91	1390	0.88
1400	0.85	1420	0.80	1440	0.76	1460	0.62	1500	0.40

Total Runoff = 6.410 Acre-ft.
 Peak Q = 83.90 CFS
 Time to Peak = 1156 Minutes

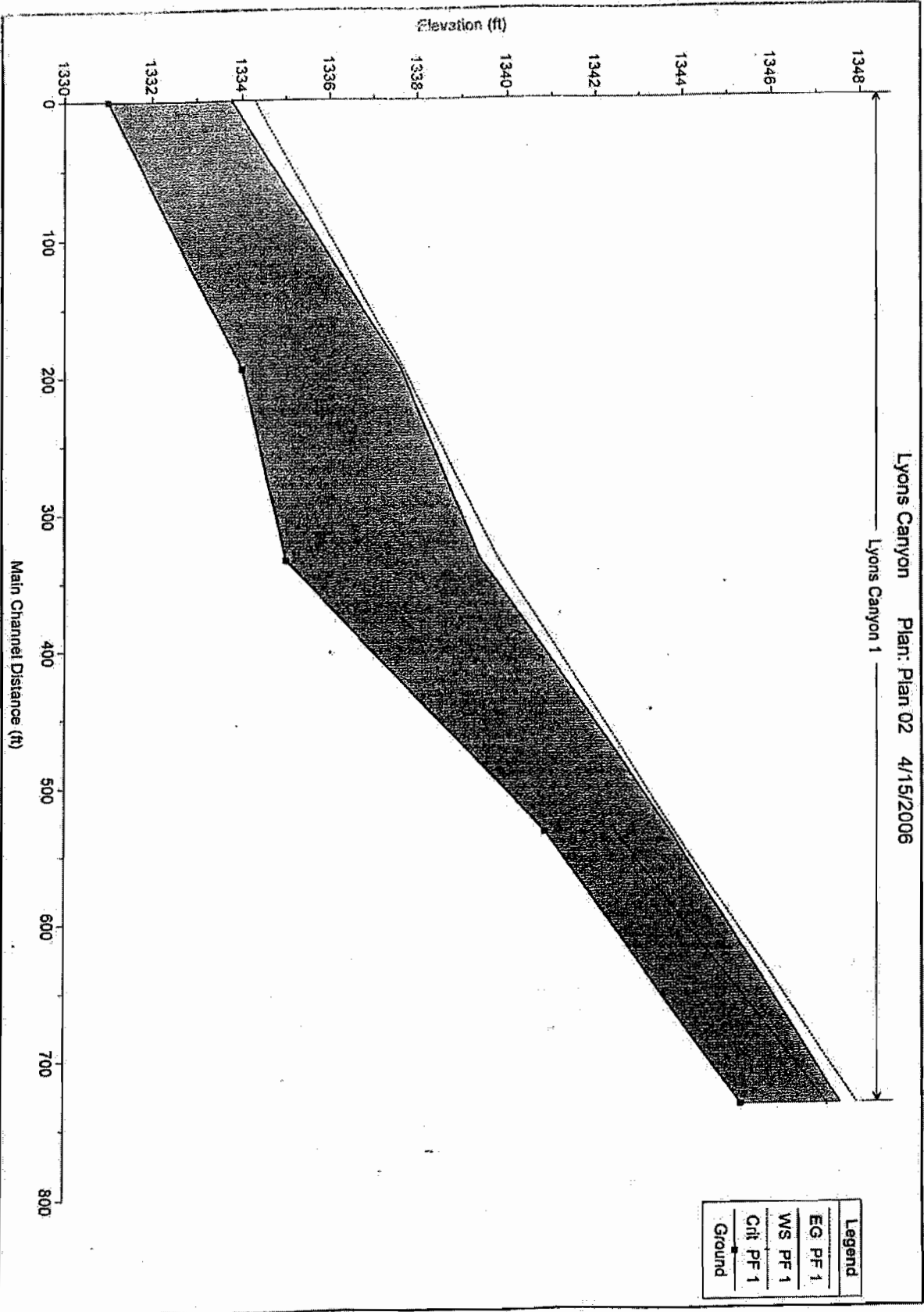
MODIFIED RATIONAL METHOD HYDROLOGY
 TOTAL HYDROGRAPH AT 1 148XZ STORM DAY 4 STORM FREQ. 50 REDUCTION FACTOR = 1.000

TIME	Q	TIME	Q	TIME	Q	TIME	Q	TIME	Q
0	1.20	100	1.59	200	1.65	300	1.71	400	1.80
500	1.91	600	2.66	700	2.25	800	2.58	900	3.47
1000	5.74	1050	7.78	1100	12.24	1110	13.75	1120	15.94
1130	19.70	1131	19.58	1132	19.93	1133	20.42	1134	20.88
1135	21.59	1136	21.92	1137	22.48	1138	23.09	1139	23.75
1140	24.47	1141	25.23	1142	26.11	1143	27.06	1144	28.13
1145	29.29	1146	30.61	1147	32.00	1148	33.53	1149	35.27
1150	37.32	1151	39.79	1152	42.85	1153	46.64	1154	51.54
1155	58.88	1156	78.69	1157	95.17	1158	98.80	1159	99.07
1160	95.75	1161	90.80	1162	82.89	1163	74.13	1164	64.25
1165	44.89	1166	28.58	1167	24.17	1168	20.99	1169	19.34
1170	17.69	1171	16.42	1172	15.31	1173	14.42	1174	13.66
1175	13.01	1176	12.43	1177	11.83	1178	11.29	1179	10.80
1180	10.36	1181	9.95	1182	9.58	1183	9.26	1184	8.95
1185	8.57	1186	8.41	1187	8.17	1188	7.94	1189	7.73
1190	7.54	1191	7.35	1192	7.18	1193	7.02	1194	6.87
1195	6.72	1196	6.58	1197	6.43	1198	6.26	1199	6.09
1200	5.93	1201	5.76	1202	5.62	1203	5.47	1204	5.35
1205	5.22	1206	5.10	1207	4.97	1208	4.85	1209	4.75
1210	4.65	1211	4.55	1212	4.45	1213	4.36	1214	4.27
1215	4.19	1216	4.11	1217	4.03	1218	3.96	1219	3.88
1220	3.82	1221	3.75	1222	3.69	1223	3.64	1224	3.59
1225	3.53	1226	3.48	1227	3.43	1228	3.39	1229	3.34
1230	3.30	1231	3.25	1232	3.21	1233	3.17	1234	3.13
1235	3.09	1236	3.05	1237	3.01	1238	2.98	1239	2.94
1240	2.91	1241	2.87	1242	2.84	1243	2.81	1244	2.78
1245	2.75	1246	2.71	1247	2.69	1248	2.66	1249	2.63
1250	2.61	1251	2.56	1252	2.53	1253	2.50	1254	2.51
1255	2.49	1256	2.48	1257	2.47	1258	2.45	1259	2.44
1260	2.41	1261	2.42	1262	2.40	1263	2.39	1264	2.39
1265	2.37	1266	2.36	1267	2.35	1268	2.33	1269	2.33
1270	2.31	1271	2.30	1272	2.29	1273	2.28	1274	2.27
1275	2.26	1276	2.25	1277	2.24	1278	2.23	1279	2.23
	2.20	1281	2.21	1282	2.20	1283	2.19	1284	2.19
	2.17	1286	2.17	1287	2.16	1288	2.15	1289	2.14
	2.14	1291	2.13	1292	2.12	1293	2.11	1294	2.10
	2.10	1296	2.09	1297	2.08	1298	2.07	1299	2.07
	2.07	1310	2.02	1320	1.96	1330	1.91	1340	1.87
1350	1.83	1360	1.79	1370	1.73	1380	1.68	1390	1.64
1400	1.60	1420	1.55	1440	1.50	1460	1.35	1500	1.00

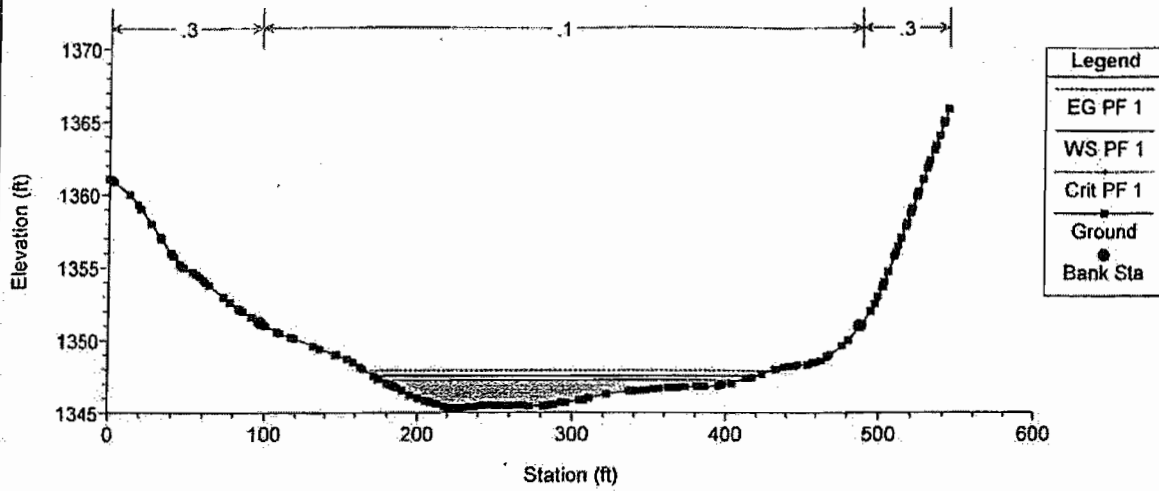
Total Runoff = 8.473 Acre-ft.
Peak Q = 99.07 CFS
Time to Peak = 1159 Minutes

Appendix 7
HEC-RAS Existing Analysis

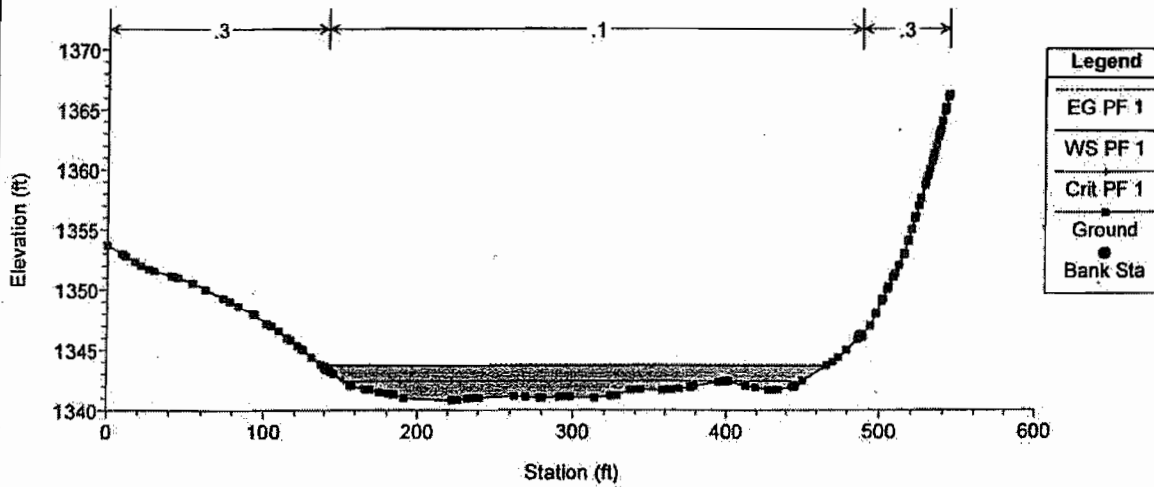
Lyons Canyon Plan: Plan 02 4/15/2006
Lyons Canyon 1



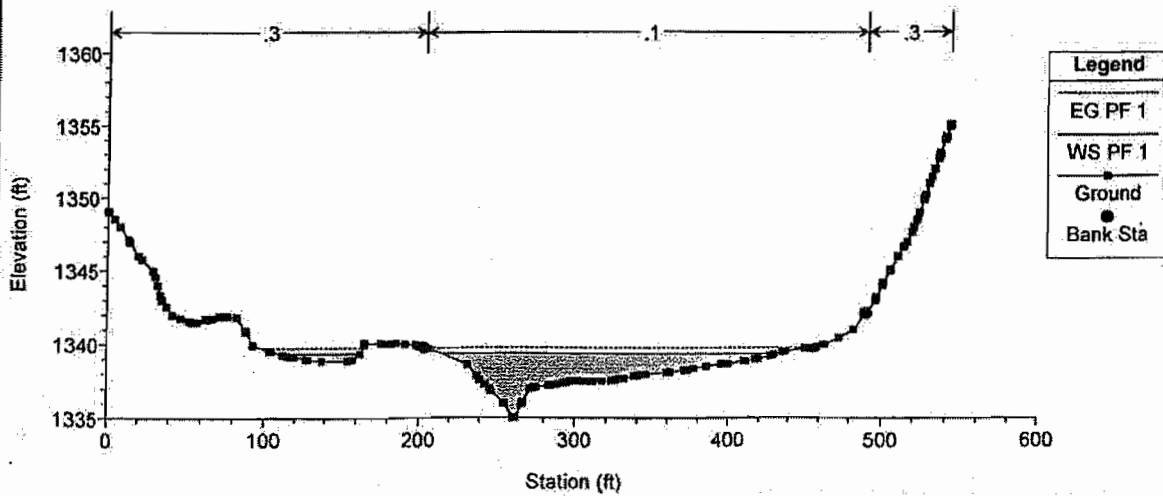
Lyons Canyon Plan: Plan 02 4/15/2006



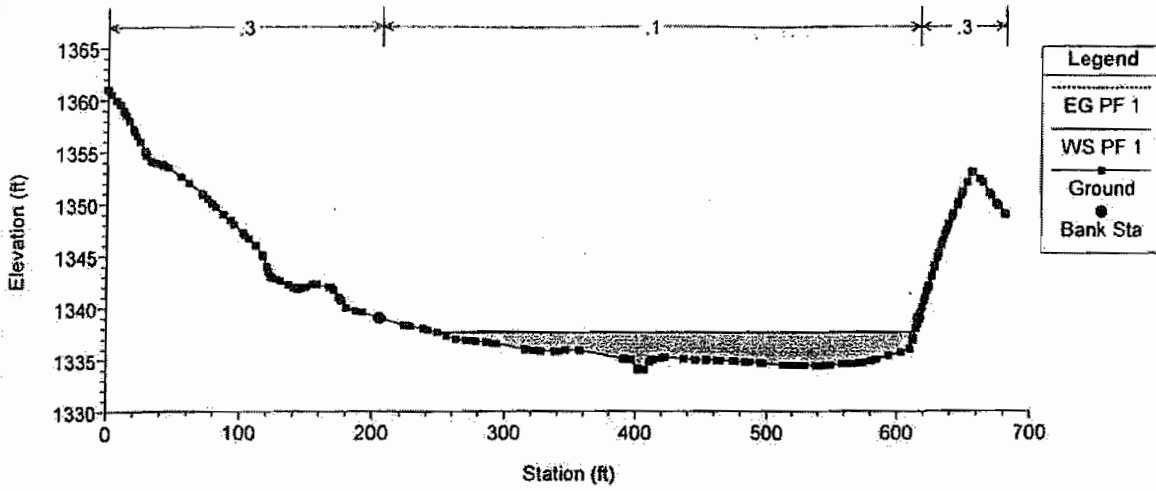
Lyons Canyon Plan: Plan 02 4/15/2006



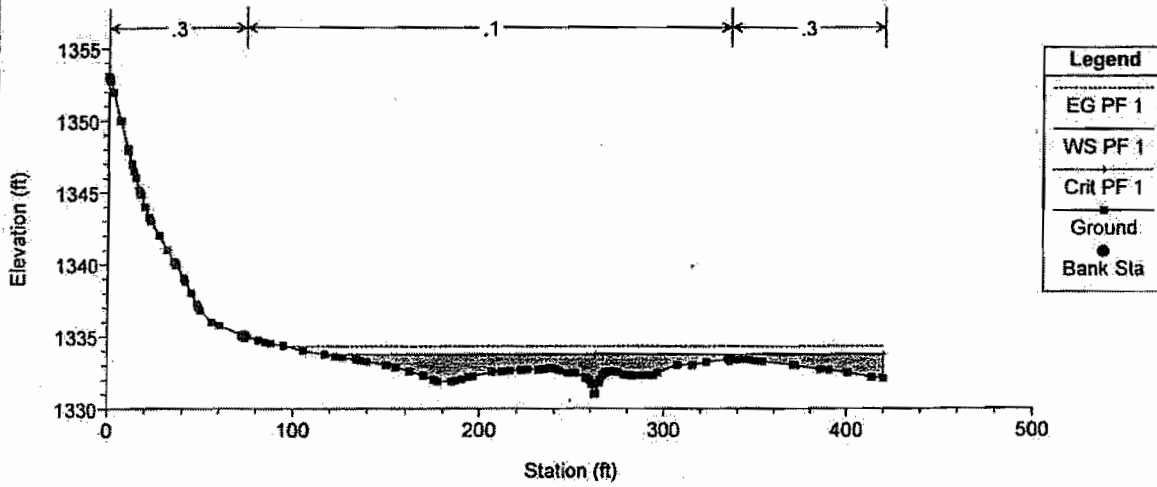
Lyons Canyon Plan: Plan 02 4/15/2006



Lyons Canyon Plan: Plan 02 4/15/2006



Lyons Canyon Plan: Plan 02 4/15/2006



HEC-RAS Version 3.1.2 April 2004
 U.S. Army Corp of Engineers
 Hydrologic Engineering Center
 609 Second Street
 Davis, California

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X   X   XXXXXX   XXXX   XXXX   XX   XXXX
X   X   X   X   X   X   X   X   X   X
X   X   X   X   X   X   X   X   X   X
XXXXXXXX XXXX   X   XXX XXXX XXXXXX XXXX
X   X   X   X   X   X   X   X   X   X
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PROJECT DATA
 Project Title: Lyons Canyon
 Project File : 149HYR00.prj
 Run Date and Time: 4/15/2006 2:15:32 PM
 Project in English units

PLAN DATA

Plan Title: Plan 02
 Plan File : C:\HEC Data\RAS\149HYR00.p02
 Geometry Title: Lyons Canyon
 Geometry File : C:\HEC Data\RAS\149HYR00.g01
 Flow Title : Flow 01
 Flow File : C:\HEC Data\RAS\149HYR00.f01

Plan Summary Information:
 Number of: Cross Sections = 5 Multiple Openings = 0
 Culverts = 0 Inline Structures = 0
 Bridges = 0 Lateral Structures = 0

Computational Information
 Water surface calculation tolerance = 0.01
 Critical depth calculation tolerance = 0.01
 Maximum number of iterations = 20
 Maximum difference tolerance = 0.3
 Flow tolerance factor = 0.001

Computation Options
 Critical depth computed only where necessary
 Conveyance Calculation Method: At breaks in n values only
 Friction Slope Method: Average Conveyance
 Computational Flow Regime: Subcritical Flow

FLOW DATA

Flow Title: Flow 01
 Flow File : C:\HEC Data\RAS\149HYR00.f01

Flow Data (cfs)

River	Reach	RS	PF 1
Lyons Canyon	1	5	1625

Boundary Conditions

River	Reach	Profile	Upstream	Downstream
Lyons Canyon	1	PF 1		Normal S = 2.5

GEOMETRY DATA

Geometry Title: Lyons Canyon
 Geometry File : C:\HEC Data\RAS\149HYR00.g01

CROSS SECTION

RIVER: Lyons Canyon
 REACH: 1 RS: 5

INPUT

Description:

Station Elevation Data num= 161

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	1361.1	1.71	1361	2.81	1360.92	3.54	1360.86
18.6	1359.3	20.41	1359	26.74	1358	32.44	1357.12
33.43	1356.98	33.52	1356.96	39.52	1356	40.97	1355.77
45.92	1355.09	47.52	1355	53.04	1354.7	55.37	1354.55
59.87	1354.17	61.25	1354	63.95	1353.8	73.52	1353
83.03	1352.2	83.92	1352.13	85.97	1352	91.71	1351.62
98.61	1351.13	100.3	1351	107.62	1350.6	109.47	1350.49
118.86	1350.14	132.05	1349.6	136.47	1349.4	146.67	1349
147.66	1348.96	154.44	1348.7	158.28	1348.46	163.2	1348.13
171.87	1347.51	174.64	1347.3	180.01	1347	182.1	1346.89
185.98	1346.7	189.7	1346.51	194.81	1346.18	200.18	1346
205.8	1345.8	208.38	1345.73	211.98	1345.61	213.06	1345.61
220.02	1345.3	220.8	1345.31	222.3	1345.29	223.38	1345.29
228.53	1345.3	230.31	1345.33	235.59	1345.41	238.15	1345.4
						242.8	1345.48

Sta	n	Val	Sta	n	Val	Sta	n	Val
0	.3	73.74	1	.3	335.91			

Bank Sta: Left Right Lengths: Left Channel Right Ccoeff Contr. Expan.
 73.74 335.91 0 0 1 .3

SUMMARY OF MANNING'S N-VALUES

River: Lyons Canyon

Reach	River Sta.	n1	n2	n3
1	5	.3	.1	.3
1	4	.3	.1	.3
1	3	.3	.1	.3
1	2	.3	.1	.3
1	1	.3	.1	.3

SUMMARY OF REACH LENGTHS

River: Lyons Canyon

Reach	River Sta.	Left	Channel	Right
1	5	200	200	200
1	4	200	200	200
1	3	114.85	140	280.81
1	2	258.89	195	325.03
1	1	0	0	0

SUMMARY OF CONTRACTION AND EXPANSION COEFFICIENTS

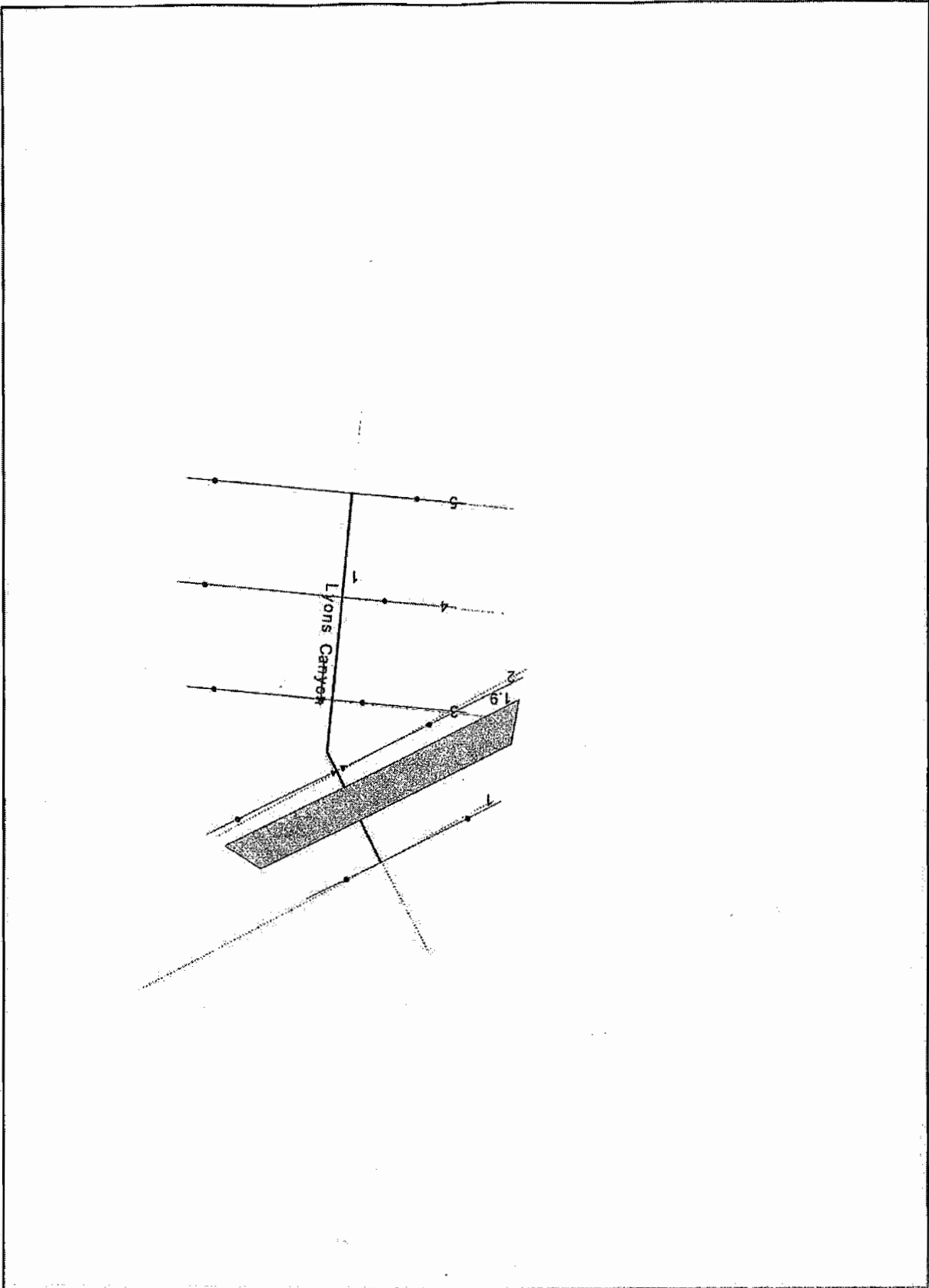
River: Lyons Canyon

Reach	River Sta.	Contr.	Expan.
1	5	.1	.3
1	4	.1	.3
1	3	.1	.3
1	2	.1	.3
1	1	.1	.3

Profile Output Table - Standard Table 1

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow
Area	Top Width	Froude #	(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq
		Chi	ft)	ft)						
1	5	PF 1	1625.00	1345.29	1347.53	1347.24	1347.89	0.070619	4.83	
336.71	249.10	0.73								
1	4	PF 1	1625.00	1340.80	1343.69	1342.42	1343.77	0.009423	2.37	
685.76	327.50	0.29								
1	3	PF 1	1625.00	1335.00	1339.39		1339.77	0.066177	4.97	
342.98	271.09	0.72								
1	2	PF 1	1625.00	1334.00	1337.62		1337.68	0.006047	1.98	
820.06	364.56	0.23								
1	1	PF 1	1625.00	1331.00	1333.76	1333.76	1334.29	0.144394	6.06	
323.41	303.88	1.01								

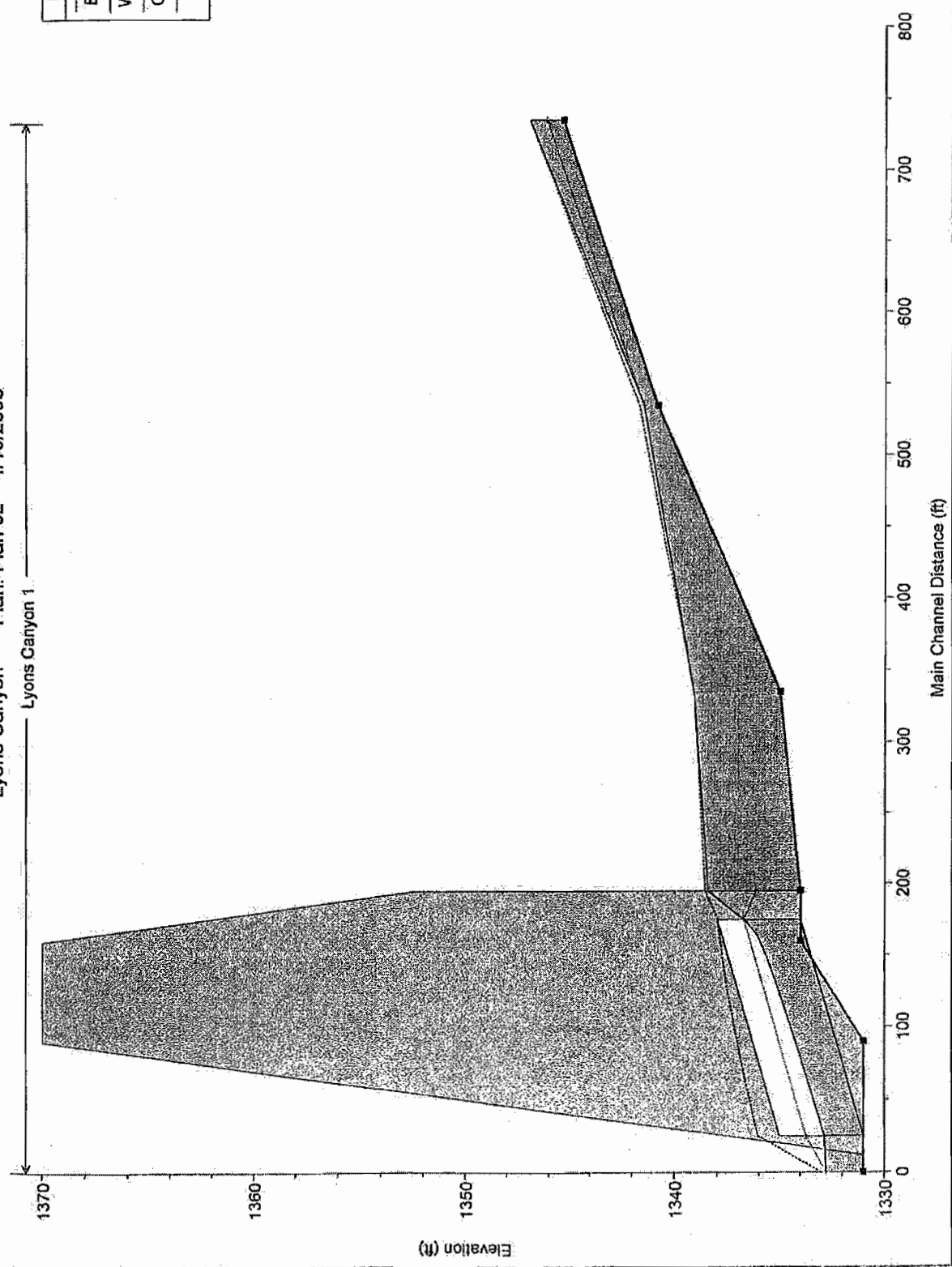
Appendix 7
HEC-RAS Proposed Analysis

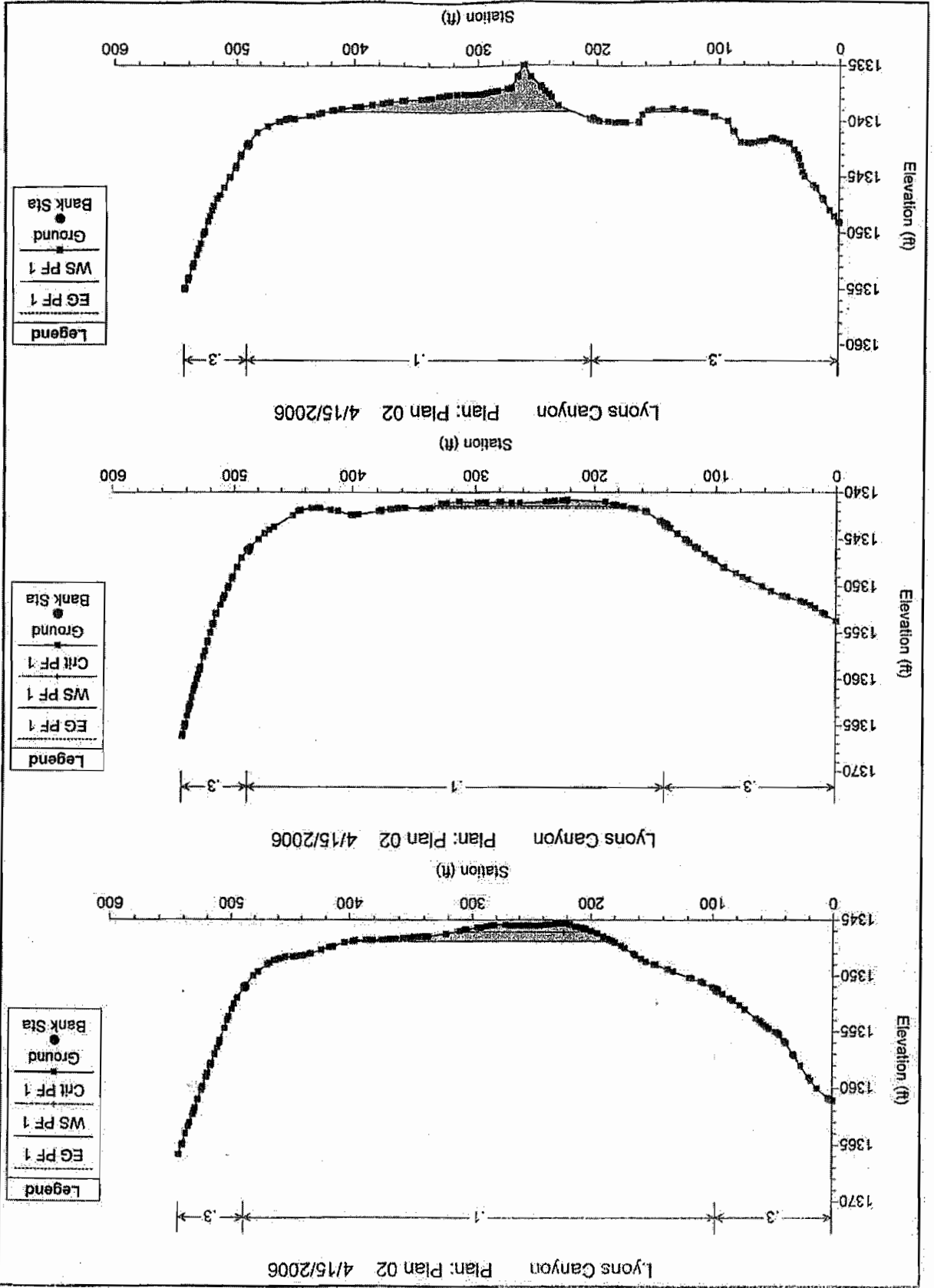


Lyons Canyon Plan: Plan 02 4/15/2006

Lyons Canyon 1

Legend	
EG PF 1	
WS PF 1	
Crit PF 1	
Ground	





Legend
 EG PF 1
 WS PF 1
 Ground
 Bank Sta

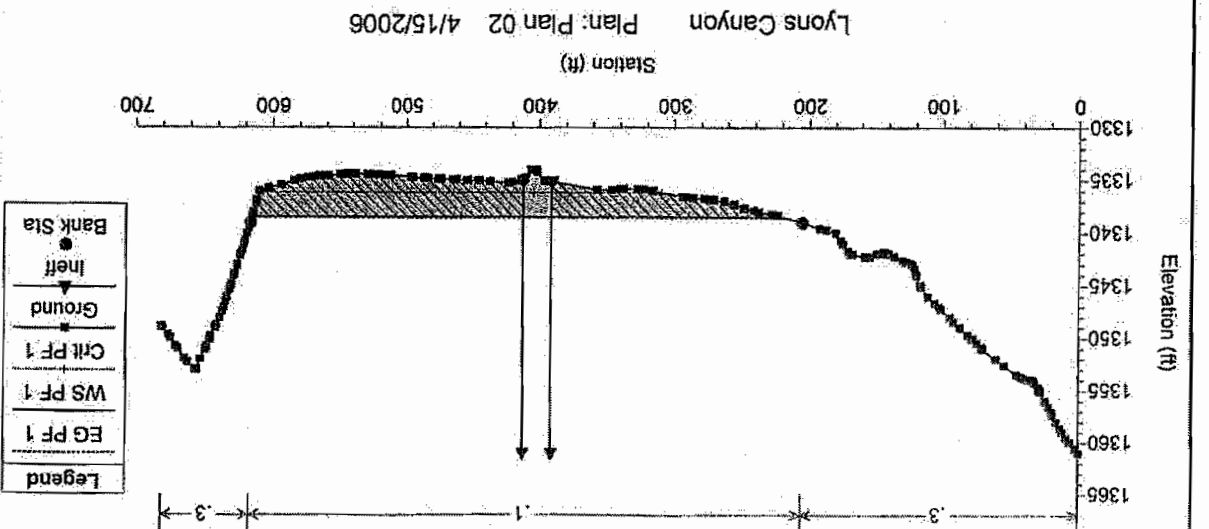
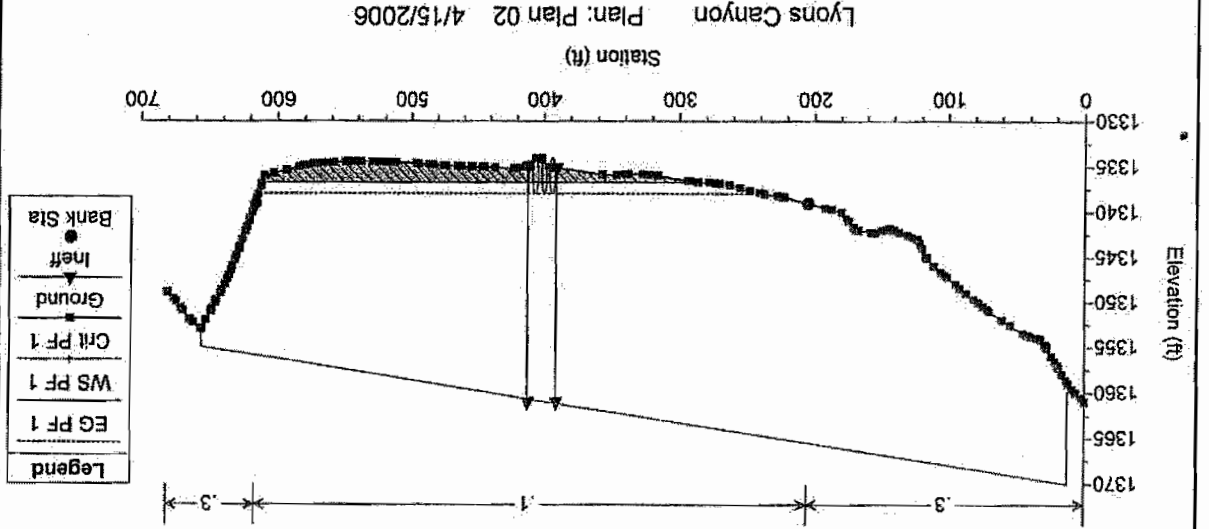
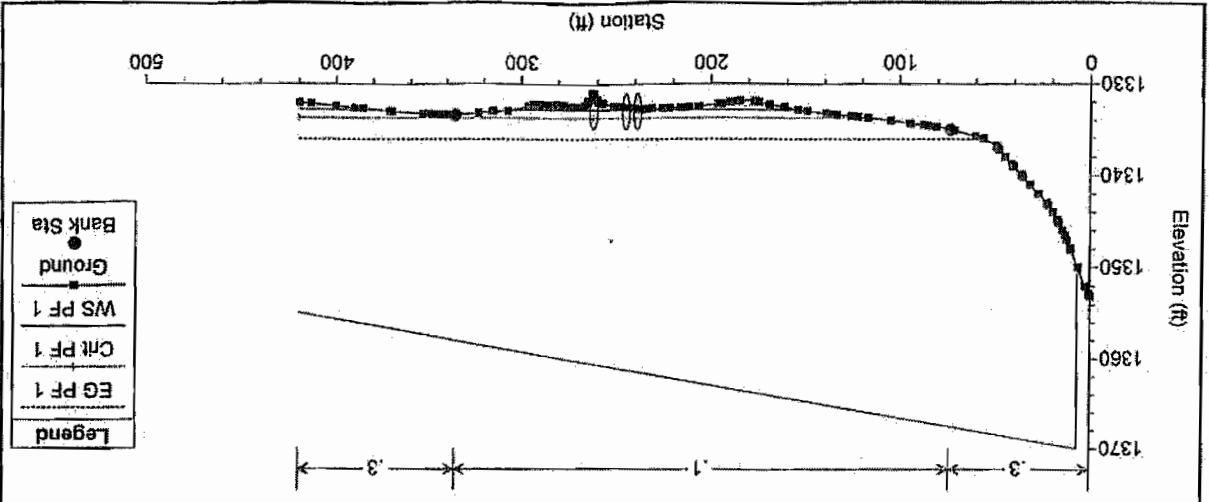
Lyons Canyon Plan: Plan 02 4/15/2006

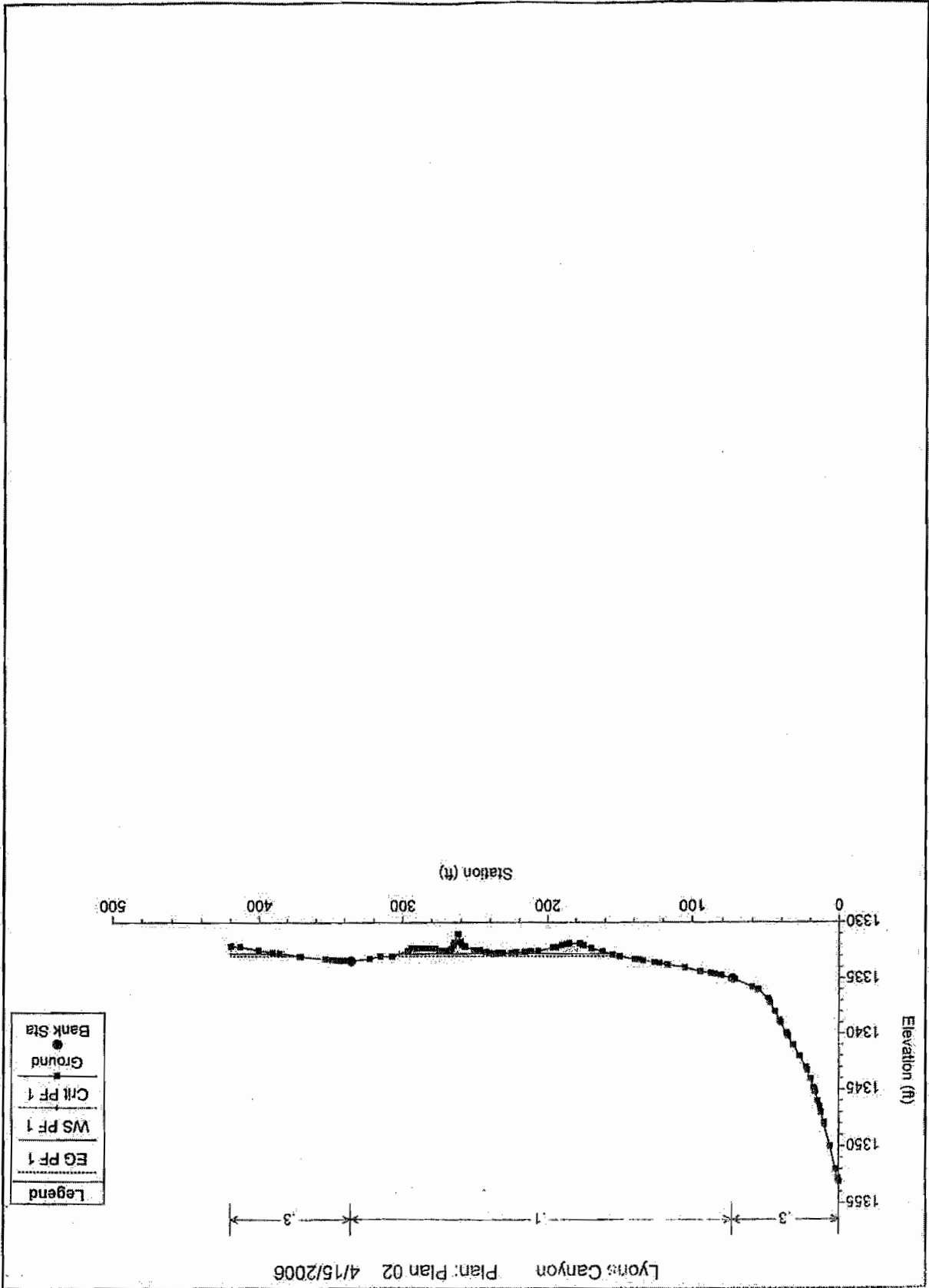
Legend
 EG PF 1
 WS PF 1
 Cnt PF 1
 Ground
 Bank Sta

Lyons Canyon Plan: Plan 02 4/15/2006

Legend
 EG PF 1
 WS PF 1
 Cnt PF 1
 Ground
 Bank Sta

Lyons Canyon Plan: Plan 02 4/15/2006





HEC-RAS Version 3.1.2 April 2004
 U.S. Army Corp of Engineers
 Hydrologic Engineering Center
 609 Second Street
 Davis, California

```

X   X   XXXXXX   XXXX   XXXX   XX   XXXX
X   X   X       X   X   X   X   X   X
X   X   X       X   X   X   X   X   X
XXXXXXXX XXXX   X   XXX XXXX XXXXXX XXXX
X   X   X       X   X   X   X   X   X
X   X   X       X   X   X   X   X   X
X   X   XXXXXX   XXXX   X   X   X   X   XXXXX
    
```

PROJECT DATA
 Project Title: Lyons Canyon
 Project File : 149HYR01.prj
 Run Date and Time: 4/15/2006 9:54:41 AM

Project in English units

PLAN DATA

Plan Title: Plan 02
 Plan File : C:\HEC Data\RAS\149HYR01.p02

Geometry Title: Lyons Canyon
 Geometry File : C:\HEC Data\RAS\149HYR01.g01

Flow Title : Flow 01
 Flow File : C:\HEC Data\RAS\149HYR01.f01

Plan Summary Information:
 Number of: Cross Sections = 5 Multiple Openings = 0
 Culverts = 1 Inline Structures = 0
 Bridges = 0 Lateral Structures = 0

Computational Information
 Water surface calculation tolerance = 0.01
 Critical depth calculation tolerance = 0.01
 Maximum number of iterations = 20
 Maximum difference tolerance = 0.3
 Flow tolerance factor = 0.001

Computation Options
 Critical depth computed only where necessary
 Conveyance Calculation Method: At breaks in n values only
 Friction Slope Method: Average Conveyance
 Computational Flow Regime: Subcritical Flow

FLOW DATA

Flow Title: Flow 01
 Flow File : C:\HEC Data\RAS\149HYR01.f01

Flow Data (cfs)

River	Reach	RS	PF 1
Lyons Canyon	1	5	245

Boundary Conditions

River	Reach	Profile	Upstream	Downstream
Lyons Canyon	1	PF 1		Normal S = 2

GEOMETRY DATA

Geometry Title: Lyons Canyon
 Geometry File : C:\HEC Data\RAS\149HYR01.g01

CROSS SECTION

RIVER: Lyons Canyon
 REACH: 1 RS: 5

INPUT

Description:
 Station Elevation Data num= 161

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	1361.1	1.71	1361	2.81	1360.92	3.54	1360.86	12.9	1360
18.6	1359.3	20.41	1359	26.74	1358	32.44	1357.12	33.28	1357
33.43	1356.98	33.52	1356.96	39.52	1356	40.97	1355.77	44.71	1355.25
45.92	1355.09	47.52	1355	53.04	1354.7	55.37	1354.55	57.61	1354.38
59.87	1354.17	61.25	1354	63.95	1353.8	73.52	1353	77.52	1352.6
83.03	1352.2	83.92	1352.13	85.97	1352	91.71	1351.62	96.84	1351.26
98.61	1351.13	100.3	1351	107.62	1350.6	109.47	1350.49	116.9	1350.21
118.86	1350.14	132.05	1349.6	136.47	1349.4	146.67	1349	147.36	1348.97
147.66	1348.96	154.44	1348.7	158.28	1348.46	163.2	1348.13	164.91	1348
171.87	1347.51	174.64	1347.3	180.01	1347	182.1	1346.89	184.36	1346.8
185.98	1346.7	189.7	1346.51	194.81	1346.18	200.18	1346	204.81	1345.79
205.8	1345.8	208.38	1345.73	211.98	1345.61	213.06	1345.61	217.62	1345.43
220.02	1345.3	220.8	1345.31	222.3	1345.29	223.38	1345.29	226.23	1345.31
228.53	1345.3	230.31	1345.33	235.59	1345.41	238.15	1345.4	242.8	1345.48

Table with 10 columns of station numbers and elevations, likely representing bridge piers or abutments.

Manning's n Values table with columns for station numbers and values.

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan. table.

CROSS SECTION

RIVER: Lyons Canyon REACH: 1 RS: 2

INPUT

Description:

Station Elevation Data table with 10 columns: Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev.

Manning's n Values table with columns for station numbers and values.

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan. table.

Ineffective Flow table with columns for station numbers and flow characteristics.

CULVERT

RIVER: Lyons Canyon REACH: 1 RS: 1.9

INPUT

Description:

Distance from Upstream XS, Deck/Roadway Width, Weir Coefficient, and Upstream Bridge Cross Section Data.

Upstream Bridge Cross Section Data table with 10 columns: Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev, Sta, Elev.

176.49	1340.60	189.16	1340	187.26	1339.7	192.65	1339.59	205.51	1339
223.27	1338.34	224.41	1338.3	227.93	1338.21	237.57	1338	241.32	1337.87
248.33	1337.65	255.67	1337.3	263.15	1337	270.84	1336.86	273.87	1336.81
278.55	1336.75	286.43	1336.7	291.53	1336.59	294.02	1336.56	316.27	1336
317.11	1335.98	322.93	1335.89	327.64	1335.84	338.2	1335.8	340.72	1335.81
346.72	1335.91	357.63	1335.9	390.79	1335.1	391.89	1335.12	396.29	1335.01
396.64	1335	401.9	1334.13	402.51	1334	406.56	1334	410.94	1334.76
412.3	1335	413.69	1335	420.4	1335.15	422.69	1335.22	423.32	1335.22
437.14	1335.1	445.41	1335.01	446.16	1335	453.94	1334.96	454.38	1335
462.43	1334.92	463.37	1334.92	465.1	1334.9	474.23	1334.85	475.89	1334.83
483.87	1334.77	486.88	1334.74	495.15	1334.7	497.11	1334.64	512.51	1334.5
517.29	1334.48	521.99	1334.47	526.95	1334.44	529.83	1334.44	538.78	1334.38
542.54	1334.38	546.28	1334.4	550.2	1334.44	559.11	1334.57	562.4	1334.59
566.49	1334.58	571.78	1334.69	574.47	1334.7	580.81	1334.86	584.74	1334.99
584.95	1335	593.94	1335.41	603.47	1335.71	610.04	1336	611.87	1336.85
612.2	1337	612.45	1337.12	614.4	1338	615.36	1339.45	616.57	1339
617.67	1339.5	618.93	1340	620.31	1340.69	621.24	1341	622.64	1341.61
623.61	1342	623.96	1342.15	626.06	1343	627.85	1343.01	628.33	1344
630.2	1344.8	630.7	1345	631.45	1345.31	633.2	1346	634.15	1346.46
635.5	1347	636.86	1347.37	638.65	1348	641.09	1348.69	642.86	1349
645.09	1349.8	645.65	1350	646.01	1350.11	648.14	1350.75	648.99	1351
649.17	1351.1	652.68	1352	655.67	1352.95	655.93	1353	656.21	1353.05
656.56	1353	661.99	1352.32	664.38	1352.01	664.44	1352	664.53	1351.98
669.28	1351	671.03	1350.73	674.37	1350	676.28	1349.69	680.33	1349
681.14	1348.9								

Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
0	.3	205.51	.1	616.57	.3

Bank Sta: Left Right Coeff Contr. Expan.

205.51	616.57	.1	.3
--------	--------	----	----

Ineffective Flow

Sta L	Sta R	Elev	Permanent
0	390.79	F	F
411.82	681.14	F	F

Downstream Deck/Roadway Coordinates

Sta Hi	Cord Lo	Sta Hi	Cord Lo
6.38	1370	420.01	1355

Downstream Bridge Cross Section Data

Station	Elevation	Data	num=	118					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
0	1353.1	.08	1353.11	.31	1353	.74	1352.79	2.38	1352
6.38	1350	10.28	1348.07	10.56	1347.93	12.55	1347	13.51	1346.5
14.78	1346	16.57	1345.18	17.13	1345	17.65	1344.83	19.85	1344
22.25	1343.25	23.14	1343	27.52	1342	32.04	1341	35.62	1340.17
36.36	1340	38.72	1339.91	40.7	1339	41.52	1338.78	44.75	1338
47.92	1337.2	48.68	1337	49.65	1336.8	56.08	1336	60.44	1335.76
71.95	1335.09	73.41	1335.01	73.55	1335	73.74	1334.99	73.87	1334.89
81.17	1334.72	84.99	1334.59	88	1334.5	94.78	1334.34	108.12	1334
117.02	1333.74	122.5	1333.6	126.21	1333.52	133.95	1333.39	136.02	1333.32
139.63	1333.26	149.73	1333	149.83	1333	154.53	1332.85	161.82	1332.56
169.59	1332.27	175.04	1332	177.22	1331.87	185.13	1331.85	186.71	1331.81
190.33	1332	194.24	1332.2	196.78	1332.21	206.82	1332.52	211.75	1332.54
212.22	1332.56	216.35	1332.6	222.45	1332.63	225.83	1332.68	231.78	1332.68
234.38	1332.71	234.85	1332.7	236.44	1332.75	237.6	1332.79	237.82	1332.79
239.02	1332.75	240.67	1332.7	243.05	1332.61	247.2	1332.47	251.42	1332.46
257.27	1332.13	257.67	1332.1	257.9	1332.13	259.7	1332	260.48	1331.7
262.06	1331	262.71	1331	264.76	1331.77	265.39	1332	266.79	1332.3
267.81	1332.5	268.27	1332.47	272.26	1332.53	273.32	1332.53	274.14	1332.5
274.91	1332.49	277.9	1332.32	278.97	1332.29	280.21	1332.28	281.3	1332.3
282.28	1332.25	286.14	1332.28	289.76	1332.28	291.88	1332.3	293.68	1332.26
296.29	1332.46	307.45	1333	315.63	1333	323.17	1333.21	335.91	1333.4
340.71	1333.35	343.39	1333.37	343.69	1333.36	345.55	1333.36	349.11	1333.3
353.6	1333.26	370.76	1333.02	371.8	1333	385.79	1332.7	390.43	1332.65
400.46	1332.46	413.74	1332.15	420.01	1332.11				

Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
0	.3	73.74	.1	335.91	.3

Bank Sta: Left Right Coeff Contr. Expan.

73.74	335.91	.1	.3
-------	--------	----	----

Upstream Embankment side slope = 2 horiz. to 1.0 vertical
 Downstream Embankment side slope = 2 horiz. to 1.0 vertical
 Maximum allowable submergence for weir flow = .95
 Elevation at which weir flow begins = 1355
 Energy head used in spillway design =
 Spillway height used in design =
 Weir crest shape = Broad Crested

Number of Culverts = 1

Culvert Name	Shape	Rise	Span
Culvert #1	Circular	4	
FNMA Chart # 1	Concrete Pipe Culvert		
FNMA Scale # 1	Square edge entrance with headwall		
Solution Criteria	Highest U.S. EG		
Culvert Upstrm Dist	Length	Top n	Bottom n
20	150	.013	.013
Depth Blocked	Entrance Loss Coef	Exit Loss Coef	
0	.5	.1	

Number of Barrels = 3
Upstream Elevation = 1334

Centerline Stations

Sta.	Sta.	Sta.
394	400	406.56

Downstream Elevation = 1331

Centerline Stations

Sta.	Sta.	Sta.
239	245	262.06

CULVERT OUTPUT Profile NPF 1 Culv Group: Culvert #1

Q Culv Group (cfs)	245.00	Culv Full Len (ft)	
# Barrels	3	Culv Vel US (ft/s)	8.91
Q Barrel (cfs)	81.67	Culv Vel DS (ft/s)	14.32

4/15/2008

4/15/2006

Elev (ft) 1334.00
 Elev (ft) 1331.00
 (ft) 1.93
 (ft) 3.06
 (ft) 0.62
 (ft)
 Elev (ft)
 Elev (ft)
 Elev (ft)
 Elev (ft)
 Depth (ft)
 Depth (ft)
 Area (sq ft)
 Elev Flow (ft) 1361.18

Supercritical.

Stn	Sta	Elev	Sta	Elev
53	74	1352.79	2.38	1352
33	12.55	1347	13.51	1346.5
45	17.65	1344.83	19.85	1344
42	32.04	1341	35.62	1340.17
39	41.52	1338.78	44.75	1338
38	56.08	1336	60.44	1335.76
35	73.74	1334.99	73.87	1334.99
35	94.78	1334.36	105.12	1334
52	133.95	1333.39	136.02	1333.32
33	154.53	1332.85	161.82	1332.56
87	185.13	1331.85	186.71	1331.91
21	206.82	1332.52	211.75	1332.54
63	225.83	1332.68	231.78	1332.68
75	237.6	1332.79	237.82	1332.79
61	247.2	1332.47	251.42	1332.46
13	259.7	1332	260.48	1331.7
77	265.39	1332	266.79	1332.3
53	273.32	1332.53	274.14	1332.5
29	280.21	1332.28	281.3	1332.3
28	291.88	1332.3	293.68	1332.26
33	323.17	1333.21	335.91	1333.4
36	345.55	1333.36	349.11	1333.3
33	385.79	1332.7	390.43	1332.65

Stn	Right	Coeff	Contr.	Expan.
0	0		.1	.3

n2	n3
.1	.3
.1	.3
.1	.3
.1	.3
.1	.3

Channel	Right
200	200
200	200
140	280.81
195	325.03
0	0

EWTS

Expan.

.3
.3
.3
.3
.3

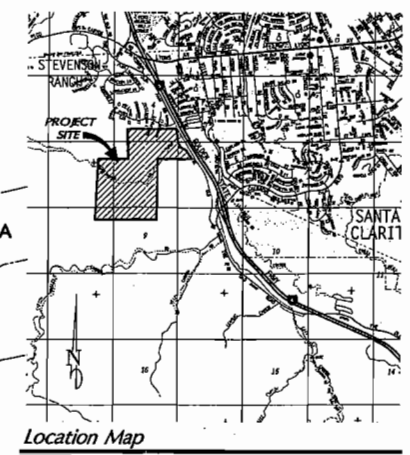
Stn	E.G. Slope	Vel Chnl	Flow
	(ft/ft)	(ft/s)	
31	0.009749	1.33	
58	0.185338	3.70	
14	0.002574	0.91	
58	0.006712	3.00	
98	0.209580	3.81	

EXISTING HYDROLOGY STUDY
TRACT 53653
 for
Lyons Canyon Ranch
 Western Pacific Housing

HYDROLOGY INFORMATION

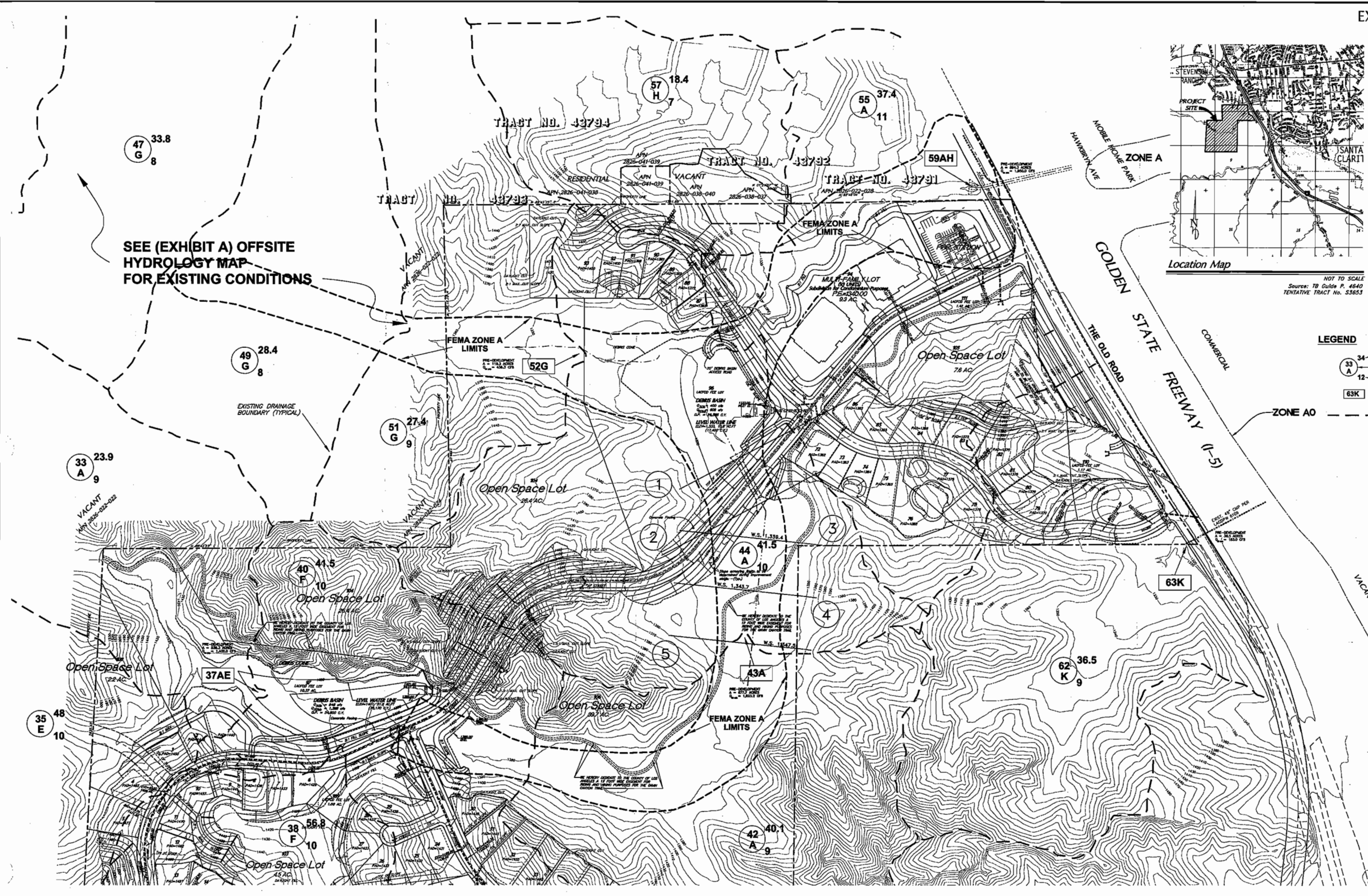
RAINFALL: ISOHYETAL METHOD
SOIL TYPE: 001, 003, 007
DESIGN FLOW: 218 CFS (UNDER 84 AC)
DESIGN PRODUCTION (P): 1.52 (UNDER 84 AC)
DESIGN FLOW (Q): 80 M, 25 YR (DEVELOPED AND CONTIGUOUS DRAINAGE)
FREQUENCY: 25 YR (DEVELOPED AND CONTIGUOUS DRAINAGE)
HYDROLOGY: 42K (RESIDENTIAL)
 18 (UNDEVELOPED)

RAINFALL INTENSITY (I) = 5.500 IN/HR (30 YR), 4.544 IN/HR (25 YR)
RUNOFF FACTOR (C) = 0.55
 $C_u = (0.8 \times 0.42) + (1.0 - 0.42) \times 0.55 = 0.685$
 $C_d = (0.85) \times (5.500) \times 0.685 = 3.21$ (DEVELOPED)
 $C_u = 1.0 - 0.55 = 0.45$
 $C_d = (0.85) \times (4.544) \times 0.45 = 1.72$ (UNDEVELOPED)
25 YR:
 $C_u = (0.8 \times 0.42) + (1.0 - 0.42) \times 0.55 = 0.685$
 $C_d = (0.85) \times (5.500) \times 0.685 = 3.21$ (DEVELOPED)
 $C_u = 1.0 - 0.55 = 0.45$
 $C_d = (0.85) \times (4.544) \times 0.45 = 1.72$ (UNDEVELOPED)
30 YR:
 $C_u = (0.8 \times 0.42) + (1.0 - 0.42) \times 0.55 = 0.685$
 $C_d = (0.85) \times (5.500) \times 0.685 = 3.21$ (DEVELOPED)
 $C_u = 1.0 - 0.55 = 0.45$
 $C_d = (0.85) \times (4.544) \times 0.45 = 1.72$ (UNDEVELOPED)



NOT TO SCALE
 Source: TO Outline P. 4840
 TENTATIVE TRACT NO. 53653

SEE (EXHIBIT A) OFFSITE
 HYDROLOGY MAP
 FOR EXISTING CONDITIONS



LEGEND

- 34 AREA (Ac)
- 33 A SUBAREA NUMBER
- 12 Tc (min.)
- 63K NODE
- ZONE A0
- - - - - EXIST. DRAINAGE BOUNDARY

DIAMOND WEST
 Engineering & Survey Services
 1000 West 10th Street, Suite 200
 Chico, California 95926
 Tel: 530-893-2700
 Fax: 530-893-2701
 E-mail: info@diamondwest.com

PLAN REVISION DESCRIPTIONS

Western Pacific Housing
 1000 West 10th Street, Suite 200
 Chico, California 95926
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 Fax: 530-893-2701
 E-mail: info@westernpacific.com

PREPARED BY OR UNDER THE DIRECTION OF

 D.W. FELT
 LICENSE NO. 51222
 CIVIL ENGINEER

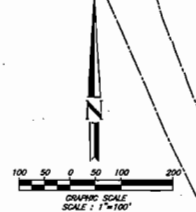
EXISTING HYDROLOGY STUDY
TRACT 53653
Lyons Canyon Ranch
 Western Pacific Housing

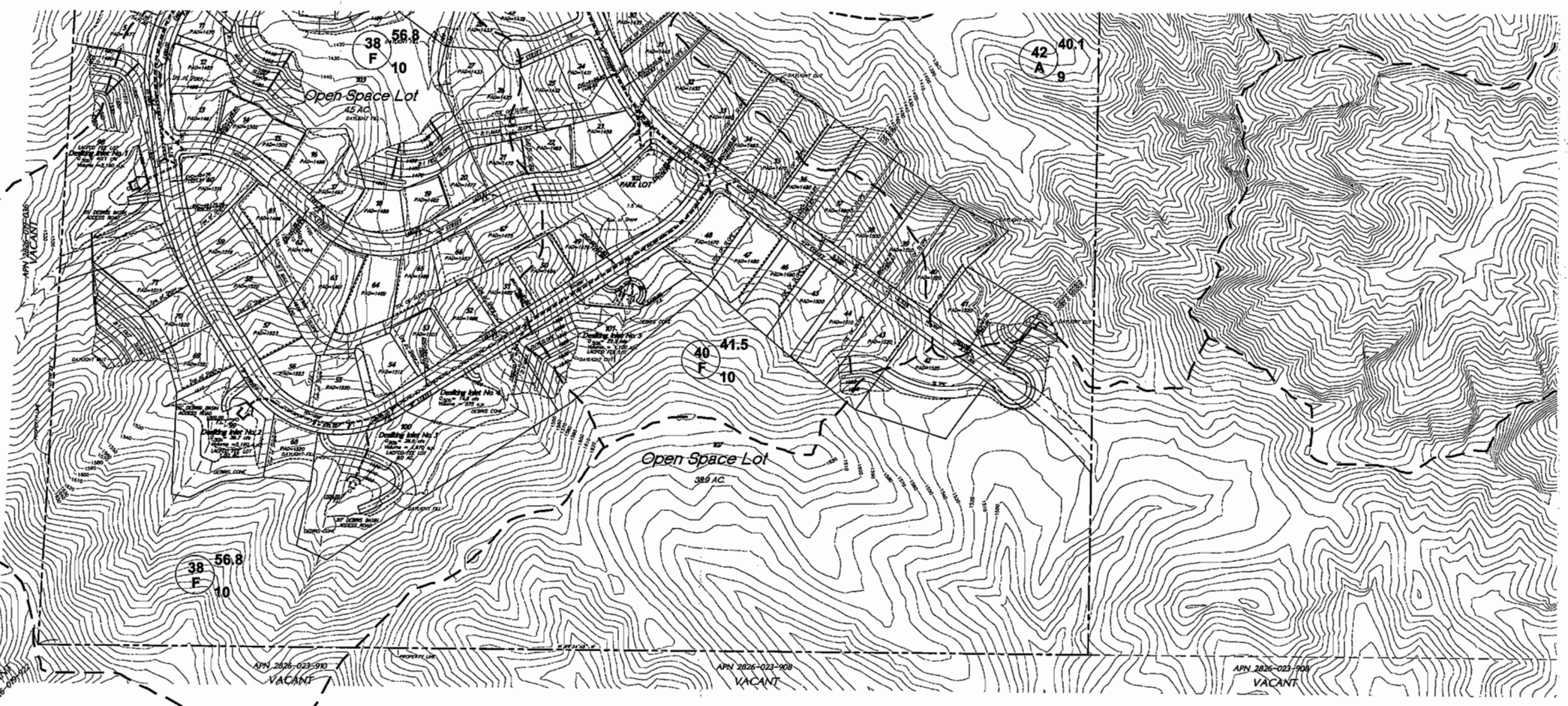
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DESIGNED:	D.W.F.
DRAWN:	D.W.F.
CHECKED:	L.S.A.
SUPERVISED:	L.S.A.
PROJ. ENGINEER:	D.W.F.

SHEET NO.

1

OF 2 SHEETS



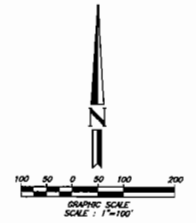


35 E 10 48

38 F 10 56.8

40 F 10 41.5

42 A 9 40.1



VACANT
APN 2826-023-908

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S.U.S.M.P. AND DRAINAGE CONCEPT PLAN

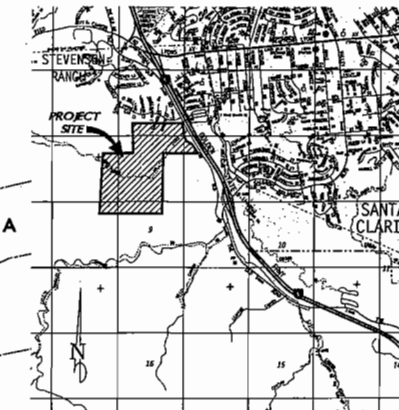
TRACT 53653
for
Lyons Canyon Ranch
Western Pacific Housing

HYDROLOGY INFORMATION

GENERAL METHOD: 1953 SCS
SOIL TYPE: 15, 16, 17, 18
DESIGN FLOW: 100-YR (15, 16, 17, 18)
DESIGN PRODUCTION (P): 15 (15, 16, 17, 18)
FLOODING RATE: 1.0 (15, 16, 17, 18)
FLOODING: 100 YR, 25 YR (DEVELOPED AND CONTIGUOUS URBAN)
15 (15, 16, 17, 18)
1X (UNDEVELOPED)

RUNOFF INTENSITY (I) = 5.500 IN/HR (25 YR), 4.944 IN/HR (25 YR)
BURNED FACTOR (BF) = 0.650
C_u = (0.8 × BF) + (1.0 - BF) × I
C_s = (0.8 × I) + (1.0 - BF) × I
C₀ = C_u / I (DEVELOPED)
C₀ = C_s / I (UNDEVELOPED)
C₀ = 1.0 (15, 16, 17, 18) (DEVELOPED & BURNED) (UNDER 84 AC.)
C₀ = 1.0 (15, 16, 17, 18) (UNDEVELOPED)

FOR SOIL TYPE 15: 30 YR
RUNOFF COEFFICIENT (C) = 0.840
C₁ = (0.8 × C) + (1.0 - C) × 0.650 = 0.885
C₂ = (0.8 × I) + (1.0 - BF) × I = 0.885
C₃ = 1.0 - 0.650 (1.0 - 0.840) = 0.609
C₄ = (0.609 × 5.500) A = 3.349 A (DEVELOPED)
C₄ = (1.433) (5.500) A = 7.882 A (DEVELOPED & BURNED) (UNDER 84 AC.)
25 YR: C₀ = 0.878 + 0.000
RUNOFF COEFFICIENT (C) = 0.833
C₁ = (0.8 × C) + (1.0 - C) × 0.650 = 0.861
C₂ = (0.8 × I) + (1.0 - BF) × I = 0.861
C₃ = 1.0 - 0.650 (1.0 - 0.833) = 0.683
C₄ = (0.683 × 5.500) A = 3.757 A (DEVELOPED)
C₄ = (1.433) (5.500) A = 7.882 A (DEVELOPED & BURNED) (UNDER 84 AC.)
80 YR: C₀ = 0.812 + 0.000
C₁ = (0.8 × C) + (1.0 - C) × 0.650 = 0.833
C₂ = (0.8 × I) + (1.0 - BF) × I = 0.833
C₃ = 1.0 - 0.650 (1.0 - 0.812) = 0.693
C₄ = (0.693 × 5.500) A = 3.811 A (DEVELOPED)
C₄ = (1.433) (5.500) A = 7.882 A (DEVELOPED & BURNED) (UNDER 84 AC.)



Location Map

NOT TO SCALE
Source: TR Guide P. 4640
TENTATIVE TRACT NO. 53653

DESILTING INLET NO. 8
ΣA = 6.0 AC
Q_{50%} = 20.5 cfs
Desilt Inlet No. 8
Volume = 1,315 c.y.

DESILTING INLET NO. 7
ΣA = 3.5 AC
Q_{50%} = 14.6 cfs
Desilt Inlet No. 7
Volume = 875 c.y.

DESILTING INLET NO. 6
ΣA = 6.7 AC
Q_{50%} = 26.6 cfs
Desilt Inlet No. 6
Volume = 1,625 c.y.

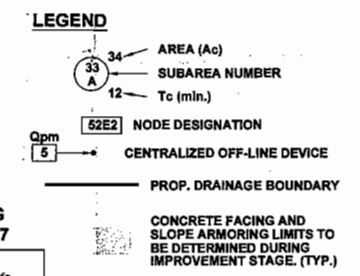
DESILTING INLET NO. 5
ΣA = 1.7 AC
Q_{50%} = 6.8 cfs

DESILTING INLET NO. 4
ΣA = 5.2 AC
Q_{50%} = 22.1 cfs

DESILTING INLET NO. 3
ΣA = 5.2 AC
Q_{50%} = 22.1 cfs

DESILTING INLET NO. 2
ΣA = 27.4 AC
Q_{50%} = 84.4 cfs

DESILTING INLET NO. 1
ΣA = 54.2 AC
Q_{50%} = 150.2 cfs



DIAMOND WEST
Engineering & Surveying

Western Pacific Housing

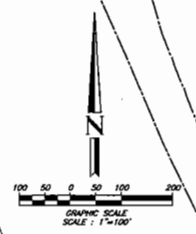
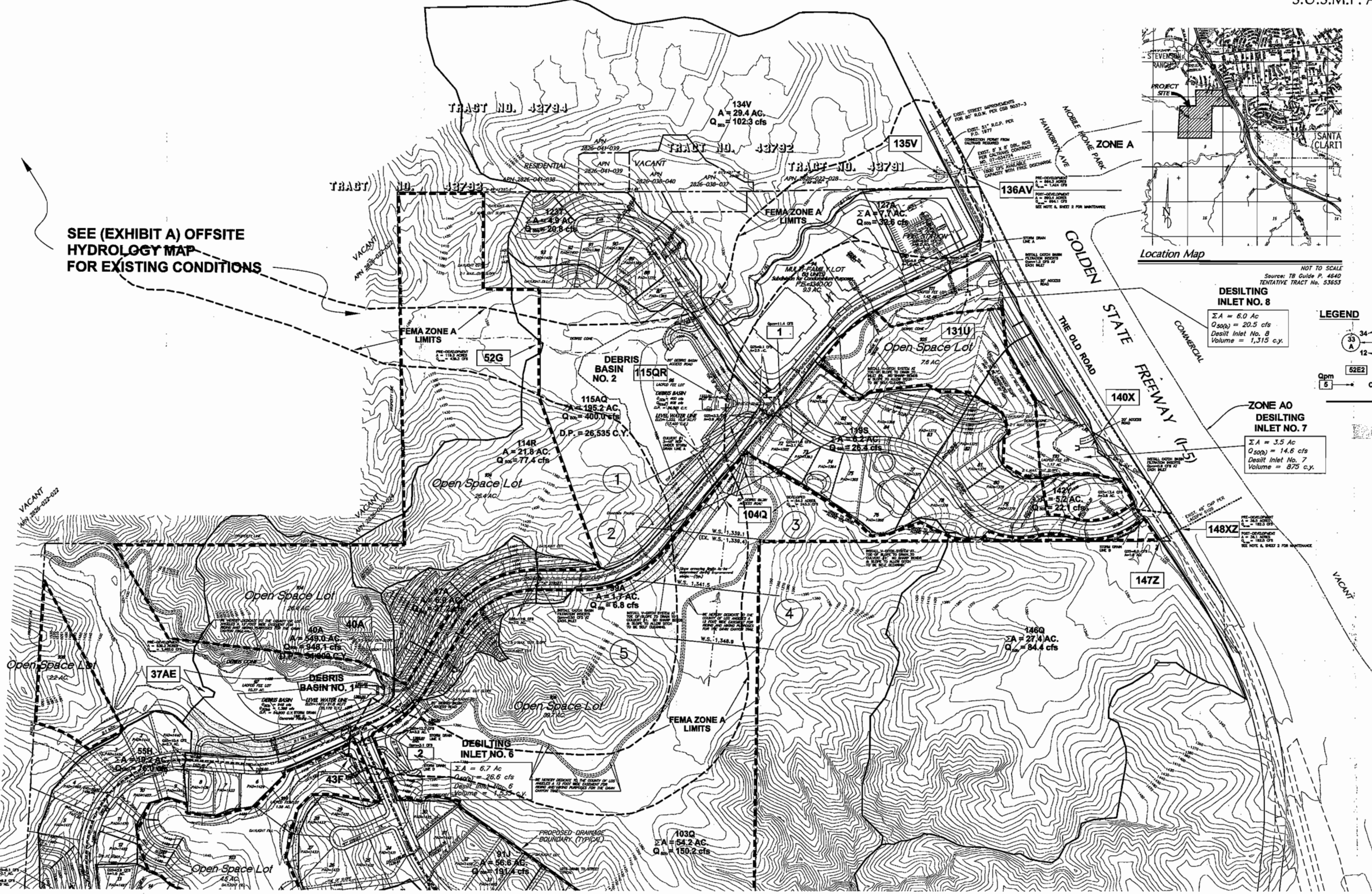
PREPARED BY OR UNDER THE DIRECTION OF

S.U.S.M.P. AND DRAINAGE CONCEPT PLAN
TRACT 53653
Lyons Canyon Ranch
Western Pacific Housing

DESCRIPTION:	BY:
DESIGNED:	D.W.E.L.
DRAWN:	D.W.E.L.
CHECKED:	L.S.A.
SUPERVISED:	L.S.A.
PROJ. ENGINEER:	B.A.L.
DRAWING SCALE:	1"=100'
DRAWING NUMBER:	6000-148

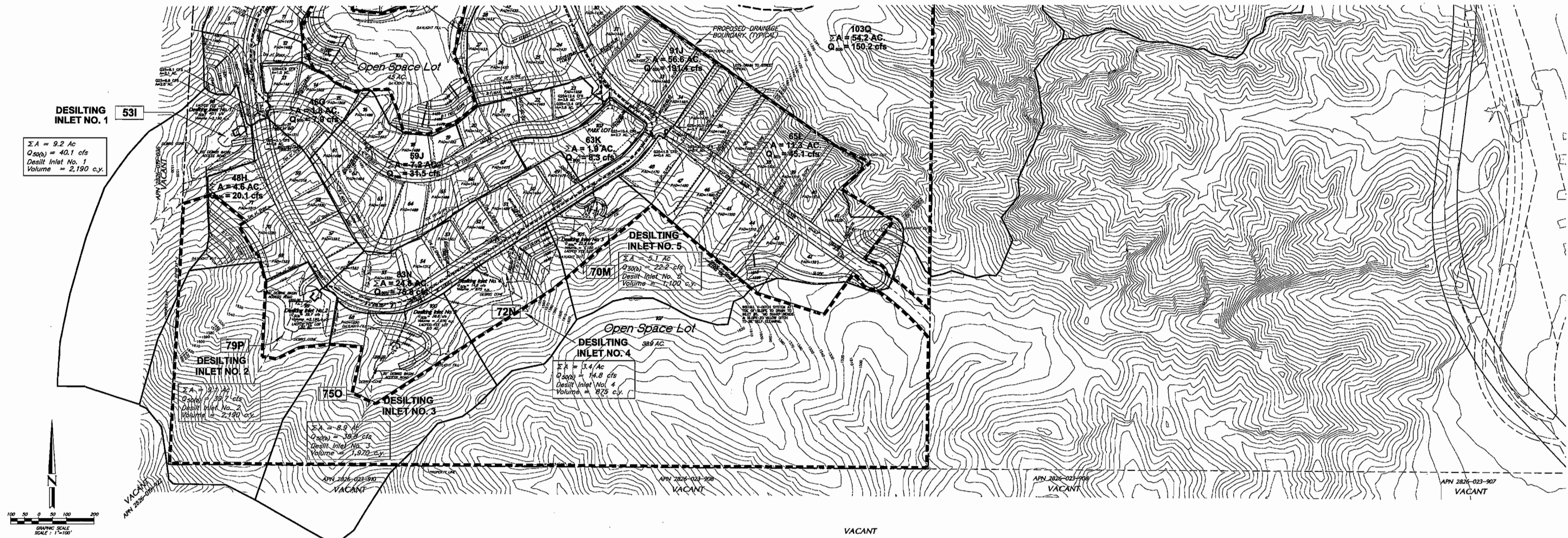
SHEET NO.
1

OF 2 SHEETS



SEE (EXHIBIT A) OFFSITE
HYDROLOGY MAP
FOR EXISTING CONDITIONS

S.U.S.M.P. AND DRAINAGE CONCEPT PLAN
TRACT 53653
for
Lyons Canyon Ranch
Western Pacific Housing



DESILTING INLET NO. 1
531
 $\Sigma A = 9.2$ AC
 $Q_{50}(p) = 40.1$ cfs
Desilt Inlet No. 1
Volume = 2,190 c.y.

DESILTING INLET NO. 5
70M
 $\Sigma A = 5.1$ AC
 $Q_{50}(p) = 22.2$ cfs
Desilt Inlet No. 5
Volume = 1,100 c.y.

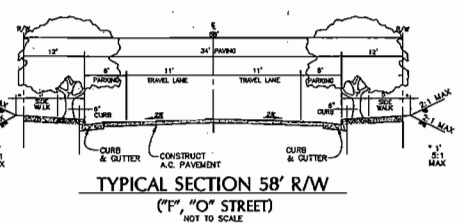
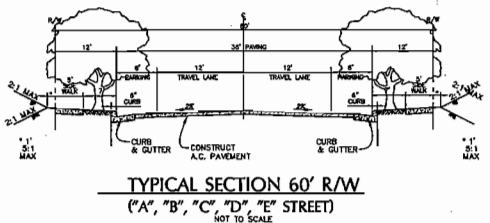
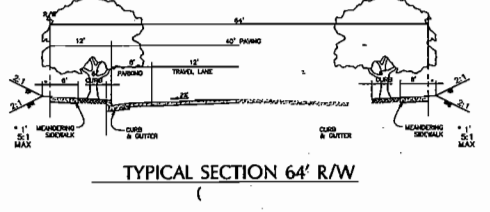
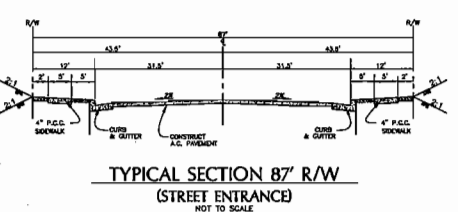
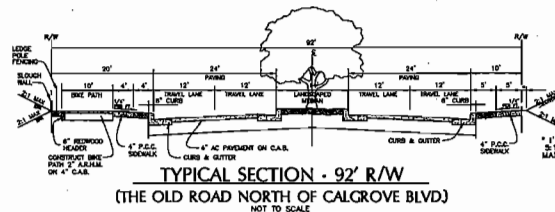
DESILTING INLET NO. 4
70M
 $\Sigma A = 5.1$ AC
 $Q_{50}(p) = 22.2$ cfs
Desilt Inlet No. 4
Volume = 1,100 c.y.

DESILTING INLET NO. 2
79P
 $\Sigma A = 9.1$ AC
 $Q_{50}(p) = 39.7$ cfs
Desilt Inlet No. 2
Volume = 2,180 c.y.

DESILTING INLET NO. 3
750
 $\Sigma A = 8.9$ AC
 $Q_{50}(p) = 39.8$ cfs
Desilt Inlet No. 3
Volume = 1,970 c.y.

- DRAINAGE CONCEPT NOTES**
1. HYDROLOGY INFORMATION AND STORM DRAIN ALIGNMENTS SHOWN ARE NOT NECESSARILY APPROVED.
 2. COMPLIANCE OF ALL STREET DRAINAGE REQUIREMENTS WILL BE MET TO THE SATISFACTION OF THE DEPARTMENT OF PUBLIC WORKS.
 3. NECESSARY EASEMENTS WILL BE SECURED FOR THE STORM DRAIN SYSTEM TO THE SATISFACTION OF THE DEPARTMENT OF PUBLIC WORKS.
 4. REGULAR ACCESS WILL BE PROVIDED TO ALL INLETS AND OUTLETS TO THE SATISFACTION OF THE DEPARTMENT OF PUBLIC WORKS.
 5. APPROVAL OF THE DRAINAGE CONCEPT DOES NOT CONSTITUTE DETERMINATION THAT THE OPPOSITE APPROVED ARE REQUIRED WITHIN THE MEANING OF GOVERNMENT CODE SECTION 94622.4 (EXCEPT AS NOTED).
 6. AN OPPOSITE DRAINAGE CONCEPT FOR ACCEPTANCE OF DRAINAGE (AND DRAINAGE FACILITIES) MAY BE REQUIRED WHERE INDICATED.
 7. A NOTE OF FLOOD HAZARD WILL BE REQUIRED WHERE INDICATED OF THIS PLAN.
 8. LAID TO MAINTAIN ALL DEBRIS BASINS AND DESILTING INLETS. CAUTIONS TO MAINTAIN PUBLIC RIGHT-OF-WAY FACILITIES LOCATED IN THE OLD ROAD. HOA TO MAINTAIN ALL OTHER ON-SITE DRAINAGE FACILITIES.
 9. HOA TO MAINTAIN ALL OTHER ON-SITE DRAINAGE FACILITIES.
 10. A SOIL REPORT WILL BE REQUIRED TO VERIFY THAT A 7-DAY PERCOLATION RATE CAN BE OBTAINED.
 11. OPEN DRAINAGE LOSSES TO MAINTAIN NATURAL CONTOURS.
 12. ALL OPEN CHANNELS TO BE DESIGNED TO AVOID SHARP BENDS AND BE SELF-CLEANING.
 13. ALL MANUFACTURED SLOPES TO BE LANDSCAPED AND MAINTAINED BY THE HOA.
 14. ANY DEVELOPMENT IN A STATE A DESIGNATION REQUIRES A CONDITIONAL LETTER OF MAP REVISION FROM FEMA PRIOR TO ISSUANCE OF GRADING PERMITS. A LETTER OF MAP REVISION WILL BE REQUIRED PRIOR TO BUILDING OCCUPANCY.
 15. ALL DRAINAGE PLANS AND DETAILS TO BE REVIEWED AND APPROVED AT IMPROVEMENT PLAN CHECK.

- BEST MANAGEMENT PRACTICES (BMP'S) LIST**
1. LOT RAINOFF TO BE INFILTRATED FROM THE GRADED PAD AREAS THROUGH ON-SITE PERVIOUS SOLES.
 2. SLOPE PROTECTION - CONCRETE RAINOFF FROM THE TOPS OF SLOPES AND STABILIZE DISTURBED SLOPES WITH LANDSCAPING PER COUNTY STANDARD.
 3. VEGETATE SLOPES WITH NATIVE OR DROUGHT TOLERANT VEGETATION TO MANAGE EROSION.
 4. ALL CATCH BASINS AND INLETS SHALL BE STOCKED WITH "SHARPENED" CONCRETE RAINOFF NOTES & STABILIZERS FOR IMPROVED BMP STANDARDS OR AS APPROVED BY DWP.
 5. RAINOFF FROM STREETS SHALL BE COLLECTED INTO CATCH BASINS WITH FINE SCREENS TO THE PROPOSED CENTRALIZED OFF-LINE DEVICE.
 6. BMP RAIN AT THE OUTLETS OF STORM DRAINS, CULVERTS, CONDUITS TO MANAGE EROSION.
 7. SOFT BOTTOM DRAINAGE - REDUCE THE ENERGY FROM DEVELOPED AREA THROUGH PAVED BED.
 8. STREET RAINOFF IS COLLECTED INTO CATCH BASINS AND STORM DRAIN PIPES OUTLET ONTO NATURAL ALLUVIAL CHANNELS AND/OR CANYON WITH ALLOWING SEDIMENT AS INFILTRATION SURFACES.
 9. EXISTING CANYON BED - MAINTAIN THE RAINOFF FROM DEVELOPED AREA THROUGH PAVED BED.
 10. CONCRETE LEVELS TO STABILIZE PERMANENT RIVER BANK.



LEGEND

- 34 AREA (Ac)
- 12 SUBAREA NUMBER
- Tc (min.)
- 52E2 NODE DESIGNATION
- 5 CENTRALIZED OFF-LINE DEVICE
- PROP. DRAINAGE BOUNDARY
- CONCRETE FACING AND SLOPE ARMORING LIMITS TO BE DETERMINED DURING IMPROVEMENT STAGE. (TYP.)

DIAMOND WEST
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APR 28/26-023-907
VACANT

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DWG JOB NUMBER	6000-149

SHEET NO.
2
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